

# **AUTOMATED TIDAL REDUCTION OF SOUNDINGS**

**E. G. OKENWA**

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## PREFACE

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# **AUTOMATED TIDAL REDUCTION OF SOUNDINGS**

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## ABSTRACT

In Hydrographic Surveying, soundings are reduced to a chart datum established at a reference gauge station from a long period of tidal observations. Unfortunately, due to the variations in tidal characteristics from place to place, soundings can only be reduced to the chart datum within the vicinity of the gauge station. As we move away from the gauge station, it becomes necessary to obtain new information on the tidal characteristics and apply necessary corrections to the chart datum to obtain an appropriate sounding datum for reducing the soundings.

To reduce soundings means to subtract the heights of tide, at the sounding locations and at the times of soundings, from the depths sounded to obtain the depths referenced to the chosen datum. Manual reduction of soundings is a tedious aspect of the field hydrographer's list of chores. There have been some attempts to automate the tidal reductions using digitized cotidal charts.

The objective of this work has been to develop alternative approaches to automated tidal reductions, namely, using analytical cotidal models. The range ratio and time lag fields have been approximated by surfaces described by two dimensional algebraic polynomials ( $P_n(\phi, \lambda)$ ). The observed time series at a reference station has been approximated by one dimensional trigonometric polynomial

With the coefficients of these Polynomials stored in the computer, the range ratio and the time lag at any point  $(\phi_i, \lambda_i)$  in the area can readily be predicted and the height of tide at the point and at time  $t$  can be predicted from the predicted height of tide at the reference station.

Test computations, using data from the 'Canadian Tides and Current Tables, 1978' for the Bay of Fundy have been done. It has been shown that the water level ( $h$ ) at a location  $(\phi_i, \lambda_i)$  can be predicted with a standard deviation ( $\sigma_{h_i}$ ) of 0.5 m or better.

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## I. INTRODUCTION

### 1.0 Chart Datum and Other Water Levels

The hydrographic surveyor must refer all his depth and height measurements to a reference datum. This reference datum, generally called chart datum, is a low water datum which by international agreement is so low that water level will seldom fall below it. The chart datum is, for purposes of integration and consistency, normally tied to the Geodetic datum which is usually defined by the mean sea level. For example, over a period of some years, tide gauges in Canada have been tied to the Geodetic Survey of Canada Datum (G.S.C.D.) [Atlantic Tidal Power Engineering and Management Committee Report, 1969]. This geodetic datum is based on the value of the mean sea level prior to 1910 as determined from a period of observations at tide gauge stations at Halifax and Yarmouth, Nova Scotia and Father Point, Quebec on the East Coast, and at Prince Rupert, Vancouver and Victoria on the Pacific. Mean Sea Level (M.S.L.), as its name implies, is the mean level taken up by the sea. It is determined at a tide gauge station from a long period of tide observations. The geoid, which is supposed to be the datum for the heights, is defined as

"that equipotential surface which on the average coincides with the mean sea level" [Thomson, 1974]. It therefore leaves the problem of mean sea level determination to be solved in order to define a height datum.

It is not easy to determine mean sea level since the actual level of the sea is continuously changing. Wemelsfelder [1970] , in his paper titled, 'Mean Sea Level as a Fact and as an Illusion', outlined two concepts of mean sea level: the Physical concept and the Emperical concept. The Physical concept according to him 'is that of a common parlance', it is the concept used in the verbal description, "the height of the mountains above sea level". This concept has the intent to overlook every motion of the sea, it intends to say, no waves, no tides, no storm surges, no wind influences, no seasonal changes, no density anomalies, no temperature anomalies. The mean sea level is rather conceptualized as, 'a physical object existing primarily in space, the way in which the ocean spans the earth.'

The emperical concept tries to quantify the mean sea level as the mean observed water levels at a tide gauge station over a period of time. This mean level even on the same sea varies from one tide gauge location to another and varies also with different time epochs. Wemelsfelder, [1970], enumerated 33 factors influencing the variations in the mean sea level and grouped them under global, regional, local

and instrumental influences. Bomford, [1971], observed that apart from tidal forces whose mean effect over a long period should be zero, other forces cause the mean sea level to depart appreciably from an exact level (equipotential) surface. Thomson [1974], further noted that, 'the problem of determining the true physical surface of the oceans is analogous to that of using Stoke's formula for geoid determination - we would require an infinite number of tide gauges, atmospheric sensors, sea temperature and density determinations'. It appears then that mean sea level, thus the geoid, cannot be easily determined.

The various other water levels\* that can be used as a datum, or that will be relevant to the subject matter of this work, will now be briefly defined and each is illustrated in Figure 1-1.

The average of recorded values of all the high and low waters over a period is called the Mean Tide Level (M.T.L.). It is obtained more easily than mean sea level and as such is sometimes used in calculations instead of the M.S.L.

The average throughout the year of heights of high waters during the spring tides is termed Mean High Water Springs (M.H.W.S.). The average throughout the year of the heights of low water during the spring tides is called Mean Low Water Springs (M.L.W.S.).

Mean High Water Neaps (M.H.W.N.) is the average

\*see Appendix III for further details regarding definitions used in Canada.

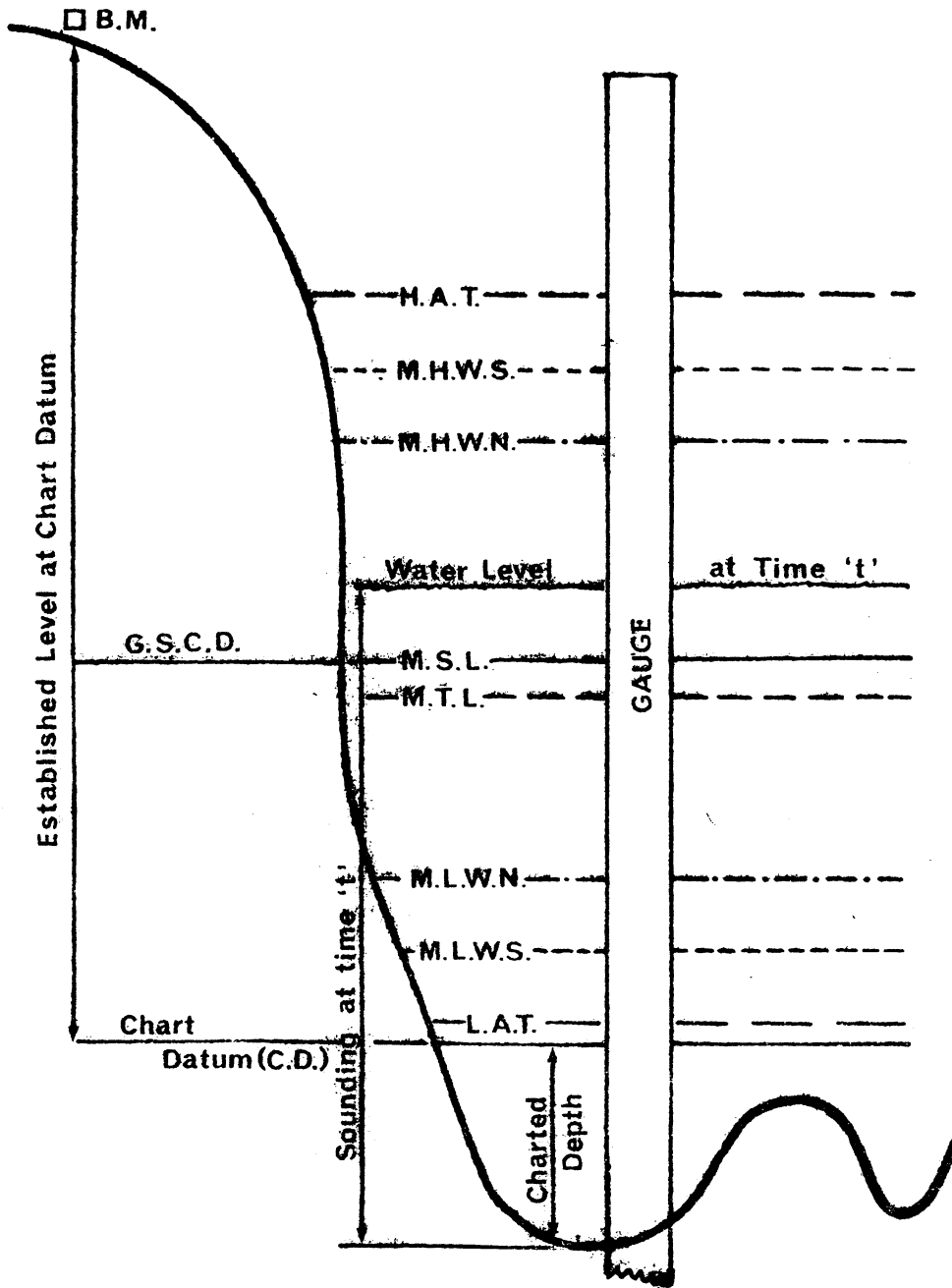


Figure 1-1

Relationship Between Various Water Levels

throughout the year of heights of high waters during the neap tides and the average throughout the year of heights of low water during the neap tides is called Mean Low Water Neaps (M.L.W.N.).

The highest tide which can be predicted to occur under average meteorological conditions and under any combination of astronomical conditions is termed Highest Astronomical Tide (H.A.T.), while the lowest predictable tide is called the Lowest Astronomical Tide (L.A.T.).

Chart datum, as previously stated, is a low water level. It is the datum to which all soundings on published charts are reduced and to which tidal predictions and tide readings are referenced. Ideally, Lowest Astronomical Tide level should be taken as chart datum. But, since we cannot accurately define it, we choose chart datum arbitrarily as close to L.A.T. as possible such that, (i) tides will seldom fall below it, (ii) it is not so low as to give unduly shallow depths.



### 1.1 Sounding Datum

When a chart datum is chosen, it can only be used within the vicinity of the gauge location [Atlantic Tidal Power Engineering and Management Committee, 1969]. Depending on the variation of tidal characteristics, it is not advisable to reduce depth measurements to this chart datum if the reference tide gauge is more than 8 km away [Admiralty Manual of Hydrographic Surveying, 1969]. This leads to the necessity of establishing a local sounding datum. In the Admiralty Manual of Hydrographic Surveying, 1969, the following rules are given as a guide to the choice of sounding datum:

- (i) if possible, a sounding datum should agree with the chart datum.
- (ii) changes in a sounding datum within the area of interest must be made whenever the nature and range of tides alter appreciably. It is difficult to lay down precise figures, but a difference in range of about one metre between two places would normally indicate the necessity for a change of datum somewhere between them.
- (iii) the time difference between tides experienced at two places will not have any effect on the difference of sounding datum between two points. It may however have a considerable effect on the

value of the reduction required to reduce soundings to datum. Therefore, it is important, even if the sounding datum does not alter, to obtain time differences between tidal stations so that time differences may be interpolated and applied to observed heights of tide used for the reduction of soundings.

- (iv) If there is any doubt in the surveyor's mind concerning the behaviour of the tide, he should set up another tide gauge to find out what is happening.

Figures 1-2 and 1-3 show how the tidal ranges change along the southern and northern coasts of the Bay of Fundy. At Yarmouth, the range at the spring tides is about 4.9 metres (16 feet). The range increases to the east and at Burnt Coat Head, a distance of about 290 km away, the range reaches about 16.7 metres (55 feet). Along the northern coast, the range is about 8.5 metres (28 feet) at Eastport, Me. and increases going eastward, and at Joggins Wharf, the range is about 12.2 metres (40 feet).

If a datum was established at Yarmouth or Eastport, Me. for the reduction of soundings, as the soundings progressed eastwards, the sounding datum should be altered. The ideal thing is to alter a sounding datum in a series of steps. Figures 1-2 and 1-3 depict the alteration of a sounding datum in steps of 0.6 m (2 feet). The correction to be

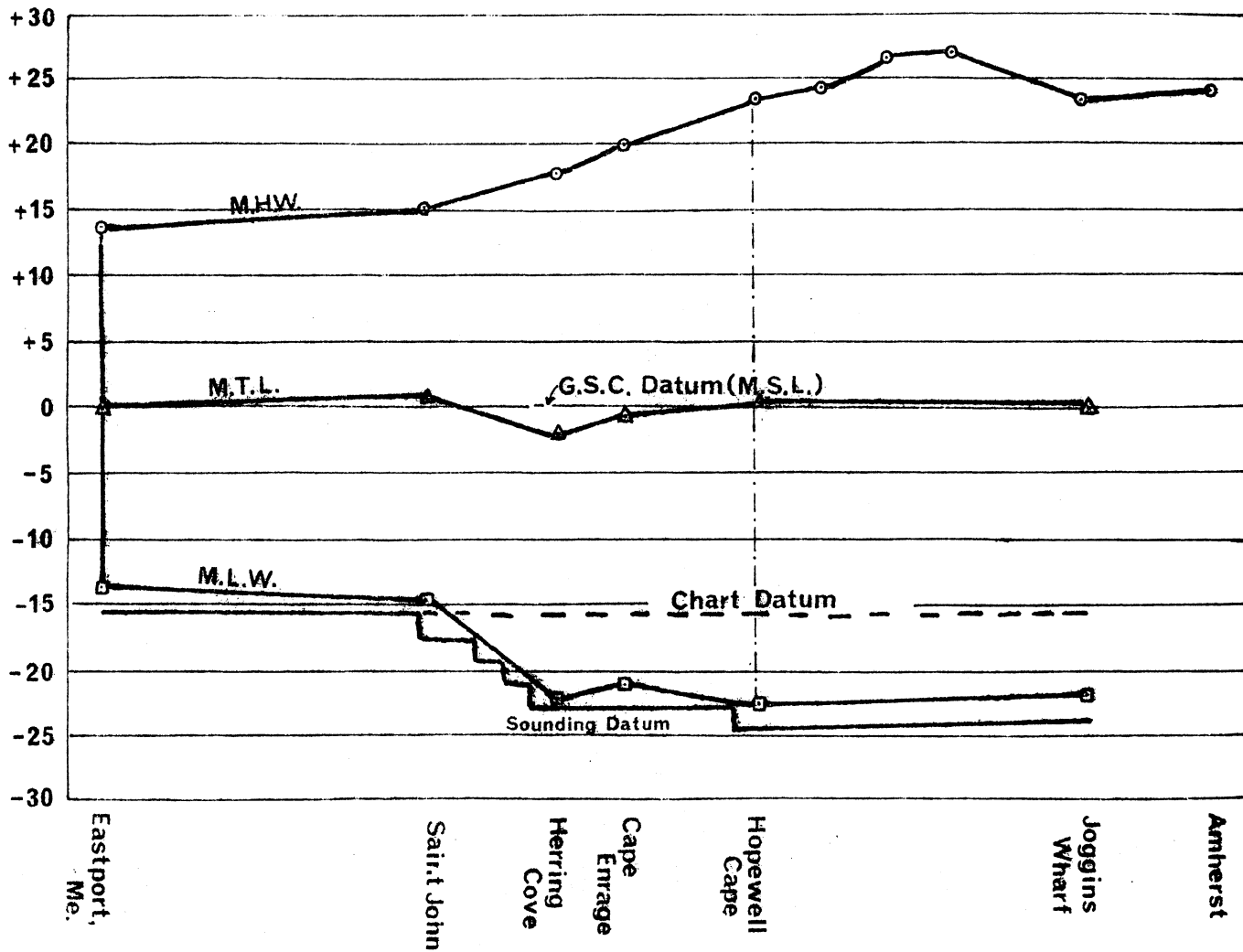


Figure 1-2

Bay of Fundy; Variation of Tidal Ranges and Sounding Datum Along the Northern Coast

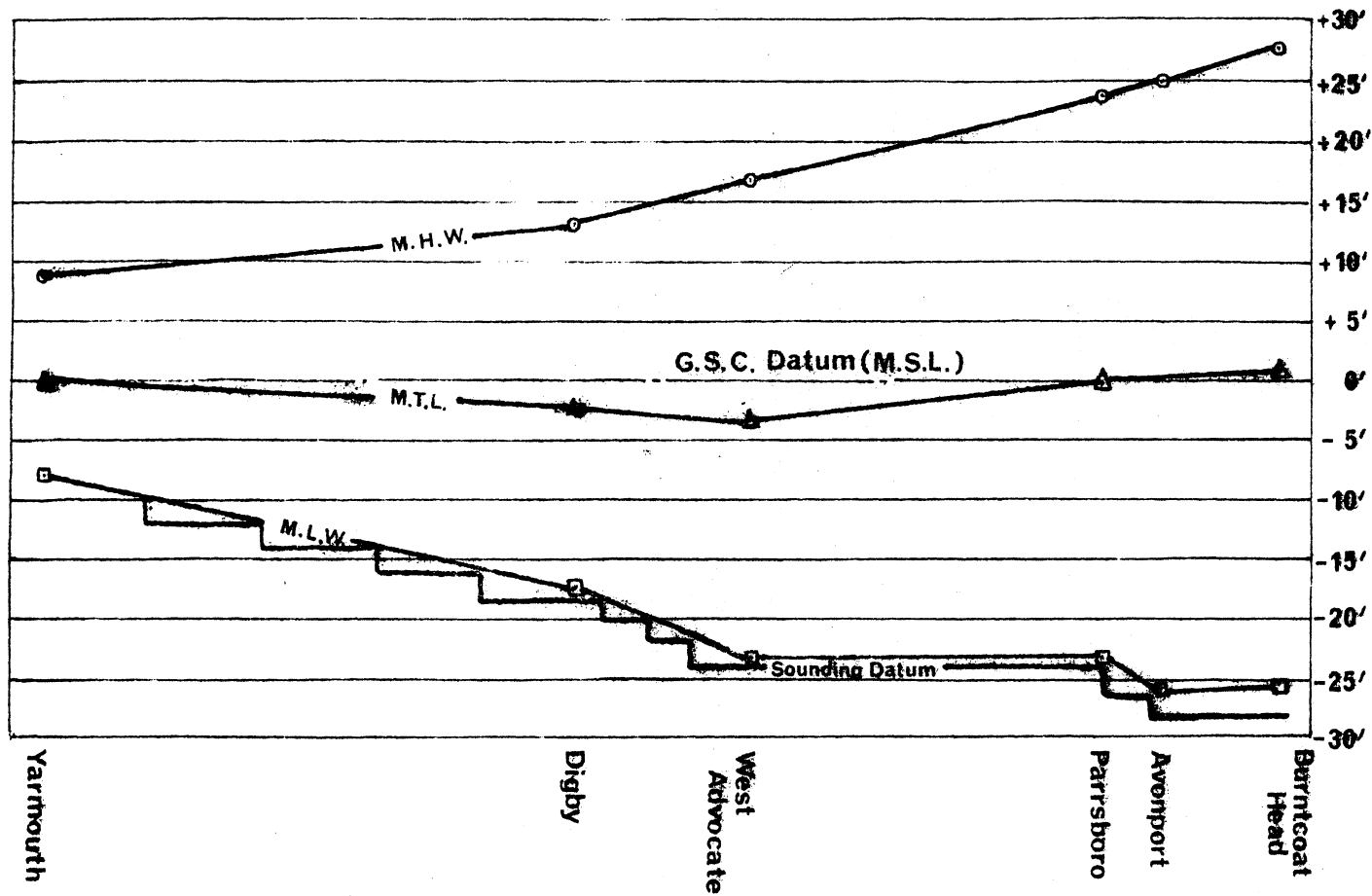


Figure 1-3  
 Bay of Fundy; Variation of Tidal Ranges and Sounding Datum Along  
 the Southern Coast

applied to a chart datum (established datum at the reference station) to obtain the sounding datum is given by [Admiralty Manual of Hydrographic Surveying, 1969],

$$d = h - H \frac{r}{R} \quad (1.1)$$

where  $h$  is the height of the M.S.L. above the zero of the new reference gauge,  $H$  is the height of the M.S.L. above the established chart datum,  $r$  is the range of tide at the new reference station and  $R$  is the range of tide at the established reference station. It means that when  $|d| \geq 0.6$  m (2 feet) the sounding datum is changed by 0.6 m (2 feet).

Figure 1-4 illustrates how a sounding datum could change in an estuary or a river. The configuration of the land and the slope of the sea bed will influence the tidal characteristics and hence the tidal ranges. The range of the tide increases at first proceeding up a river and then starts to decrease until it reaches zero at a point inland where the river ceases to be tidal.

It is not possible to establish one sounding datum for a hydrographic survey which covers a long stretch of coastline and where tidal conditions are unknown. Tidal information in the area must be built up and a sounding datum transferred gradually along the coast as the survey progresses. A hydrographic surveyor on a sounding mission could be met with any of the following situations regarding sounding datum:

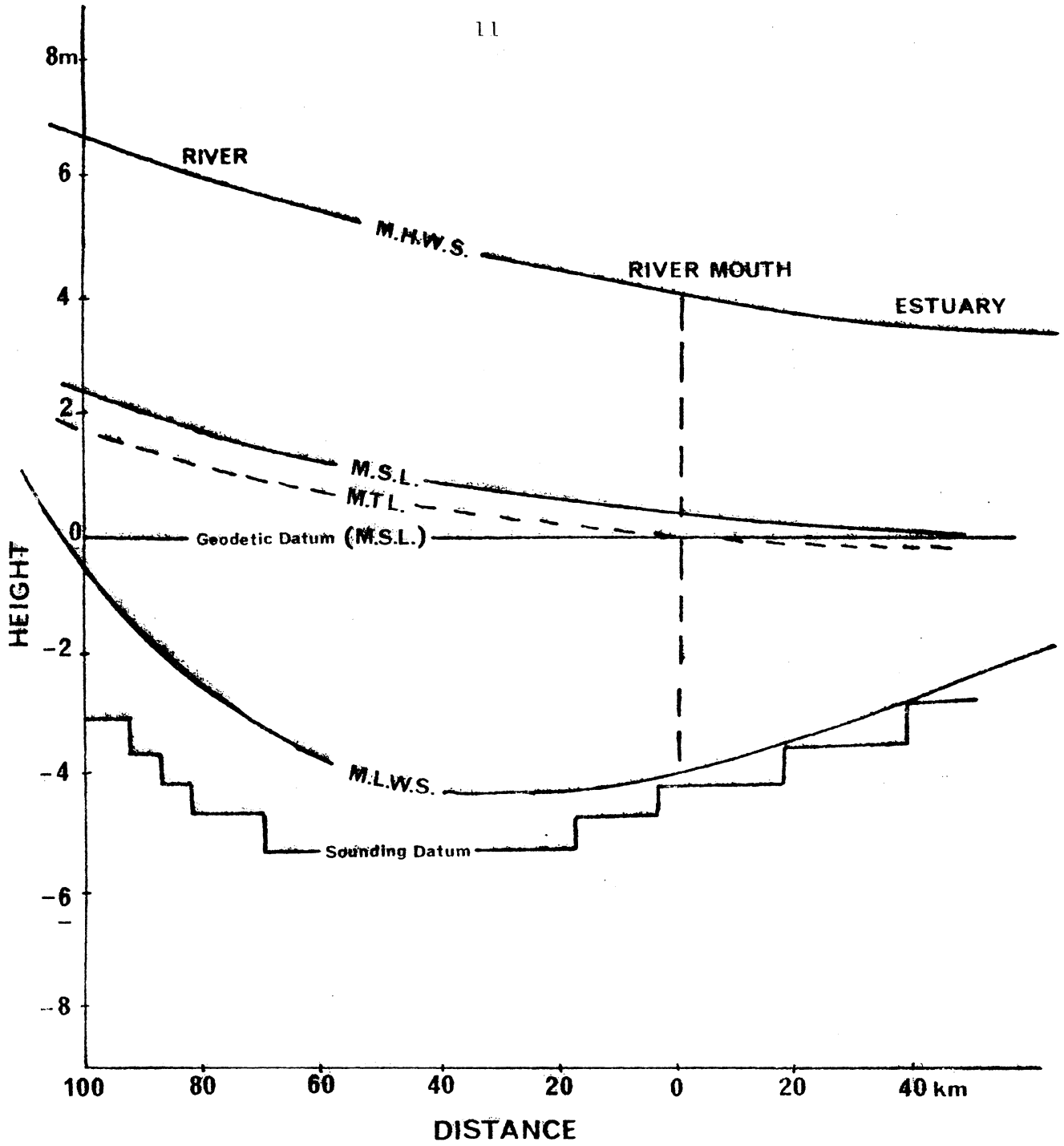


Figure 1-4

Variation of Sounding Datum in an Estuary or a River

(adapted from Admiralty Manual, 1969)

- (i) a chart datum has already been established within the sounding area,
- (ii) a chart datum has been established near the sounding area,
- (iii) a chart datum has not previously been established anywhere nearby.

The actions corresponding to the above situations are:

- (i) the surveyor should recover the established chart datum and use it,
- (ii) the surveyor should transfer the datum to the survey area; in other words, he should obtain a sounding datum for the area to be surveyed referenced to the established chart datum,
- (iii) the surveyor should aim at establishing a chart datum.

## 1.2 Reduction of Soundings

Figure 1-1 illustrates the relationship between a sounding at a time  $t$  and the chart datum. The height of tide at time  $t$  must be subtracted from the depth sounded to yield a reduced sounding. Manual reductions of soundings in tidal waters is a tedious aspect of the field hydrographer's tasks. It requires that a tide gauge be set up in the survey area and the rise and fall of tides observed while the sounding is performed. From the observed heights, it is possible to plot a curve showing the variations in the water levels and to reduce the soundings to a suitable reference plane.

Figure 1-5 illustrates a typical reduction curve [Admiralty Manual of Hydrographic Surveying, 1969]. It has been drawn from the height observations at half hourly intervals with additional readings on either side of the high water. The reductions are scaled in steps of one metre and noted in the form of a table. For example, the reduction is 5 m from 1247 hrs to 1342 hrs, 6 m from 1343 to 1446 hrs.

For inshore surveys, it is usually convenient to set up a tide gauge and observe the tides while sounding is proceeding. If we are sounding offshore, the problem becomes complicated. It may be possible to use drying banks, islets or temporary structures such as drilling rigs as sites for tide gauges. Another possibility in the near future will be the use of automatic sea bed tide gauges [DeWolfe, 1977].



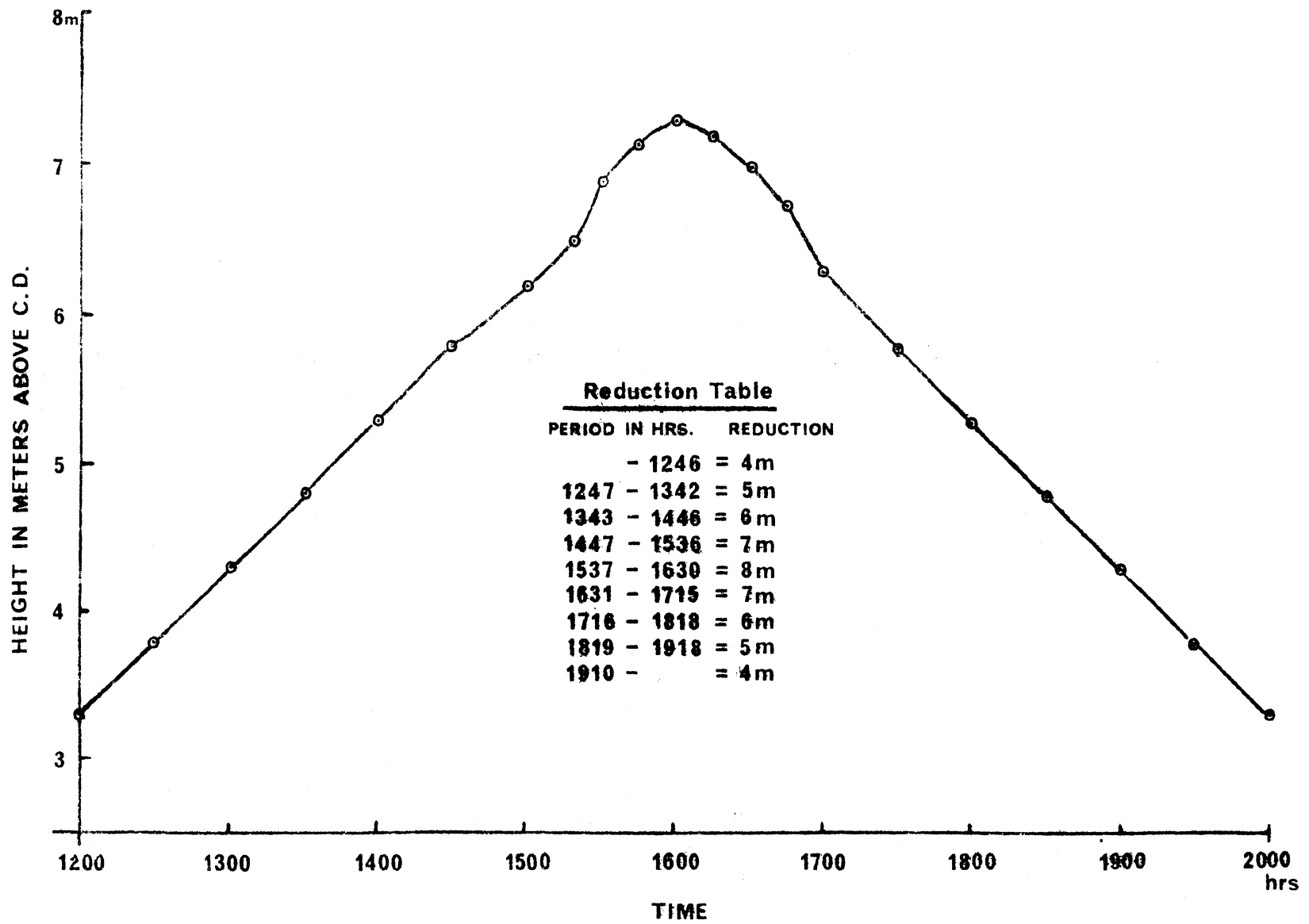


Figure 1-5

Tidal Reduction Curve

In the absence of the above alternatives, tidal observations could be made from an anchored survey vessel using an echo sounder.

If the cotidal charts for the area of interest are available or could be constructed, the necessary tidal information for the reduction of the sounding can be recovered from them. The objective of this report is to offer an automated analytical alternative to the manual task of tidal reduction of soundings through the use of tidal observations or cotidal chart information or a combination of the two.

Before describing the proposed scheme, an understanding of tidal theories and phenomena, analysis and prediction of tides, and the types and construction of cotidal charts are pertinent. Chapter II covers the theory of tide generation, harmonic analysis and prediction of tides. Chapter III is devoted to the types, construction and uses of cotidal charts.

## II ANALYSIS AND PREDICTION OF TIDES

### 2.0 Introduction

When the water levels  $h(t)$  have been observed at times  $t$  relative to a chosen datum at a tide gauge station, we have obtained a record distributed in time space (time series) and defined at the discrete time intervals. There is a trigonometric polynomial,  $P_n(t)$ , of the form

$$\tilde{h}(t) = \sum_{i=0}^n (a_i \cos \omega_i t + b_i \sin \omega_i t), \quad (2.1)$$

which can predict this time series at any time  $t$  in the interval. The analysis of this time series means the determination of the real numbers  $a_i$ ,  $b_i$ , and  $\omega_i$ . If we seek a least squares solution to this problem, we would have a system of normal equations that would be nonlinear. The presence of the non-linear trigonometric terms as unknowns leads to a serious problem which may or may not have a solution [Vaniček and Wells, 1972]. If, however, the frequencies  $\omega_i$  are known, the coefficients  $a_i$  and  $b_i$  can be determined using least squares harmonic analysis.

The first and basic problem of harmonic tidal analysis, therefore, is the determination of the constituent frequencies  $\omega_i$ . This is the first step in the complete decomposition of the observed time series into individual trigonometric terms. The first practical attempt at the determination of the constituent frequencies was made by

Darwin in 1886 using the orbital theories of the moon and the sun. In 1921, Doodson improved on the method by making a more complete expansion of the tidal potential using the modern luni-solar orbital theories.

The careful analysis of the tides at Honolulu and Newlyn by Munk and Cartwright [1966], indicated that the spectrum of a tidal record is a continuous function of frequency  $\omega$  over the low frequency band, but that it approximates closely a line spectrum over the other frequencies - 'the constituent lines emerge from the noise background as trees from grass' [Godin, 1972]. As long as we do not work with the low frequency band, (as is generally the case in Hydrographic Surveying), it is reasonable to assume that to a good order of approximation the spectrum of a tidal record is a line spectrum. We can therefore treat the observed heights as a problem of spectral analysis of a time series. Letting

$$H_k = a_k^2 + b_k^2,$$

and

$$\alpha_k = \text{Arctan}(b_k/a_k).$$

equation 2.1 can be rewritten as

$$\tilde{h}(t) = \sum_{k=0}^{\infty} H_k \cos(\omega_k t + \alpha_k), \quad (2.2)$$

where  $H_k$  is the amplitude of the constituent frequency  $\omega_k$ ,  $\alpha_k$  is the phase of the constituent at time  $t = 0$ . If the function is defined on the finite set  $M = \{0, \pm 1, \pm 2, \pm 3, \dots, \pm \ell\}$ , the frequency  $\omega_k$  is given by

$$\omega_k = \pi/\ell k . \quad (2.3)$$

$H_k$  is obviously a non negative real number that describes the magnitude of the constituent frequency  $\omega_k$ . By plotting the amplitude against integer frequencies, a visual interpretation of the contributions of the individual constituent frequencies (Figure 2-1) can be made. This represents the discrete transformation of the function from time space into frequency space [Vanicek and Wells, 1972].

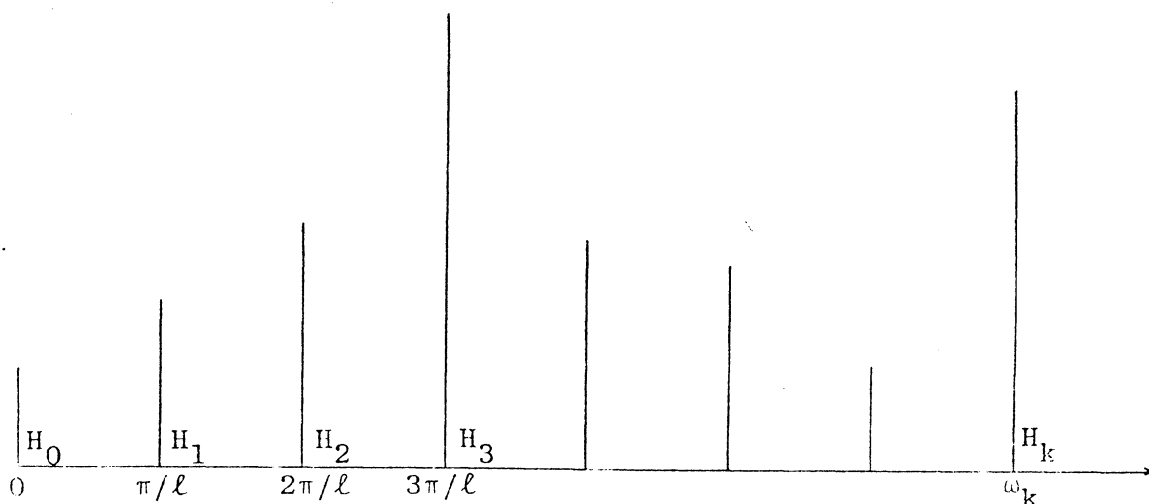


Figure 2-1

Line Spectrum of Function  $h(t)$

Munk and Cartwright, [1966] introduced an entirely different method of tidal analysis which they called the response method. In this method, the potential is generated as a time series  $V(t)$  and an attempt is made at the

prediction of height of the tide at a time  $t$  as the weighted sum of the past and present values of the potential

$$h(t) = \int_s W(s)V(t - \tau_s). \quad (2.4)$$

The weights  $W(s)$  are determined such that the prediction error  $h(t) - \tilde{h}(t)$  is a minimum in the least square sense.

In this chapter, the theory of tidal generation and the traditional harmonic analysis and prediction of tides are described. The thinking behind the response analysis and prediction is briefly outlined.

## 2.1 Theory of Tide Generation

### 2.1.1 The Movements of the Moon (Real) and the Sun (Apparent)

The moon and the sun are the principal tide generating agents. Other heavenly bodies are either too distant or have too little mass to exert any significant force on the earth's surface. Figure 2-2 shows the relationship between the orbit of the moon and the apparent orbit of the sun. The sun moves in an apparent path around the earth on a plane called the ecliptic once every 365.25 solar days. For our present purposes, this movement can be regarded as uniform and inclined at an angle of  $23^\circ 27'$  (obliquity of the ecliptic) to the celestial equator. The point where the ecliptic crosses the celestial equator from south to north (B in Figure 2-2) is called the Vernal equinox or the first point of Aries  $\tau$ .

The moon moves eastward around the earth in an orbit

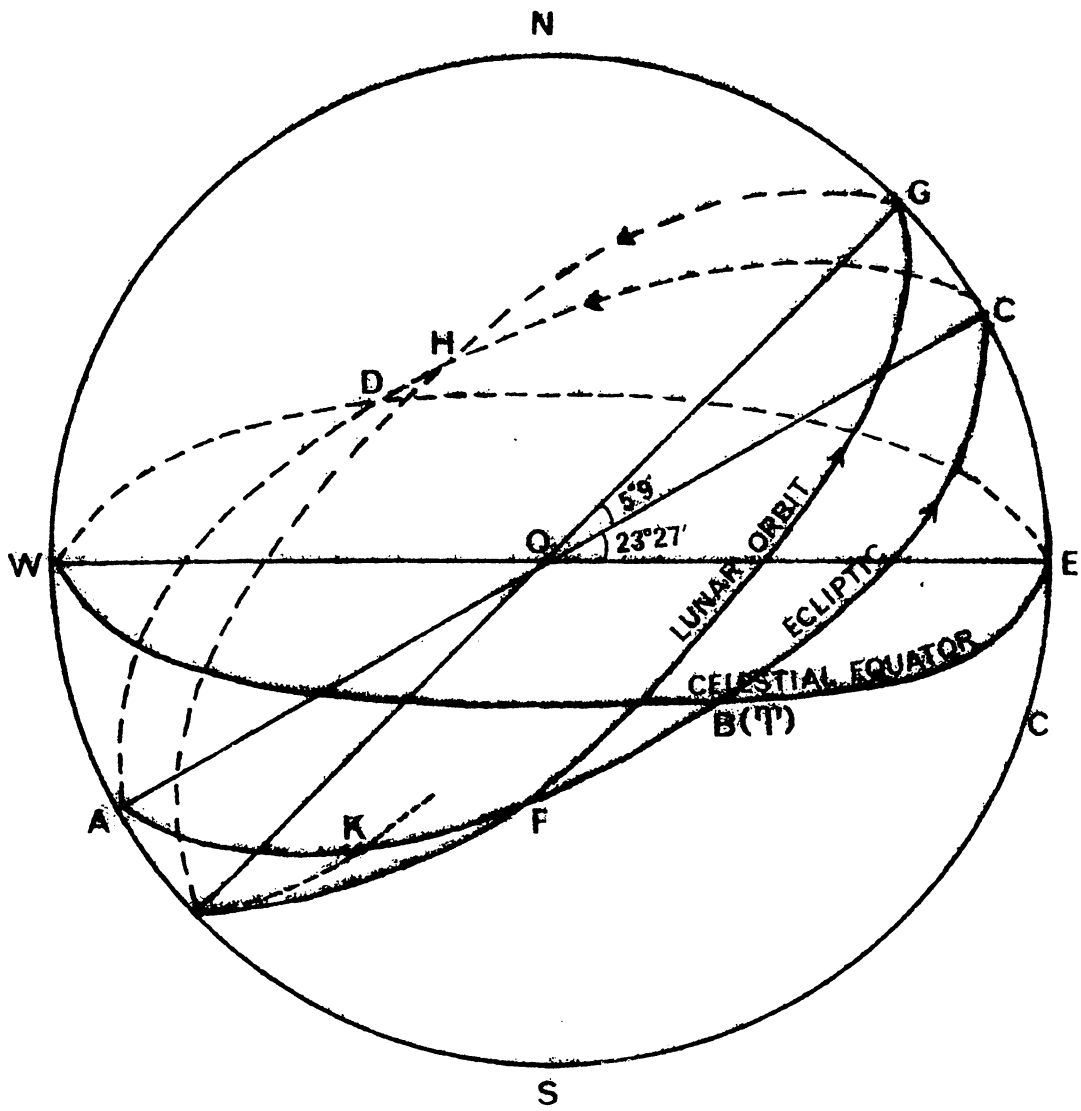


Figure 2-2

**The Relationship Between the Orbital Motions  
of the Moon and the Sun**

inclined at about  $5^{\circ} 9'$  [Admiralty Manual of Hydrographic Surveying, 1969] to the ecliptic and crosses the ecliptic at the nodes. It takes approximately 27.2122 mean solar days for the moon to travel from the ascending node F to the ascending node K (Figure 2-2). As indicated in Figure 2-2, the lunar orbit does not cross the ecliptic at the same place consecutively. The nodes continually move westward along the ecliptic and this nodal movement or regression, as it is often called, has a period of 18.61 tropical years (one tropical year = 365.2422 mean solar days). Due to the nodal regression, the obliquity of the lunar orbit with respect to the celestial equator varies progressively between a maximum and a minimum, namely,

$$\text{Max.} = 23^{\circ} 27' + 5^{\circ} 9' = 28^{\circ} 36' ,$$

$$\text{Min.} = 23^{\circ} 27' - 5^{\circ} 9' = 18^{\circ} 18' .$$

#### 2.1.2 The Tide Generating Forces and Potentials

To derive the mathematical expression for the tide generating forces of the moon and the sun, the principal factors to be taken into consideration are:

- (i) the revolution of the moon around the earth in an orbit inclined to the equator,
- (ii) the motion of the earth around the sun along the ecliptic which is also inclined to the equatorial plane,
- (iii) the rotation of the earth around its axis.



The tide generating forces at the earth's surface result from a combination of two basic forces; (i) the force of gravitation exerted by the moon (and sun) upon the earth, and (ii) centrifugal forces produced by the revolutions of the earth and the moon (and the earth and the sun) around their common centre of mass known as the barycentre.

The magnitude of centrifugal force produced by the revolution of the earth-moon system around barycentre (which lies approximately 1709 km beneath the earth's surface on the side towards the moon and along the line connecting centres of mass of the earth and of the moon) is the same at any point on or beneath the earth's surface [National Ocean Survey, 1977]. Its magnitude is [Godin, 1972]

$$F_c = KM/\rho_0^2 ,$$

where  $\rho_0$  is the distance between the centres of mass of the earth and of the moon (Figure 2-3), K is the universal gravitational constant, and M is the mass of the moon.\* The gravitational force exerted by the moon is different at different positions on or beneath the earth's surface because the force of attraction

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\*Note: The earth-moon system is used here to develop the equations for tidal potential. The same development resulting in similar equations can be used for the sun or any other heavenly body.

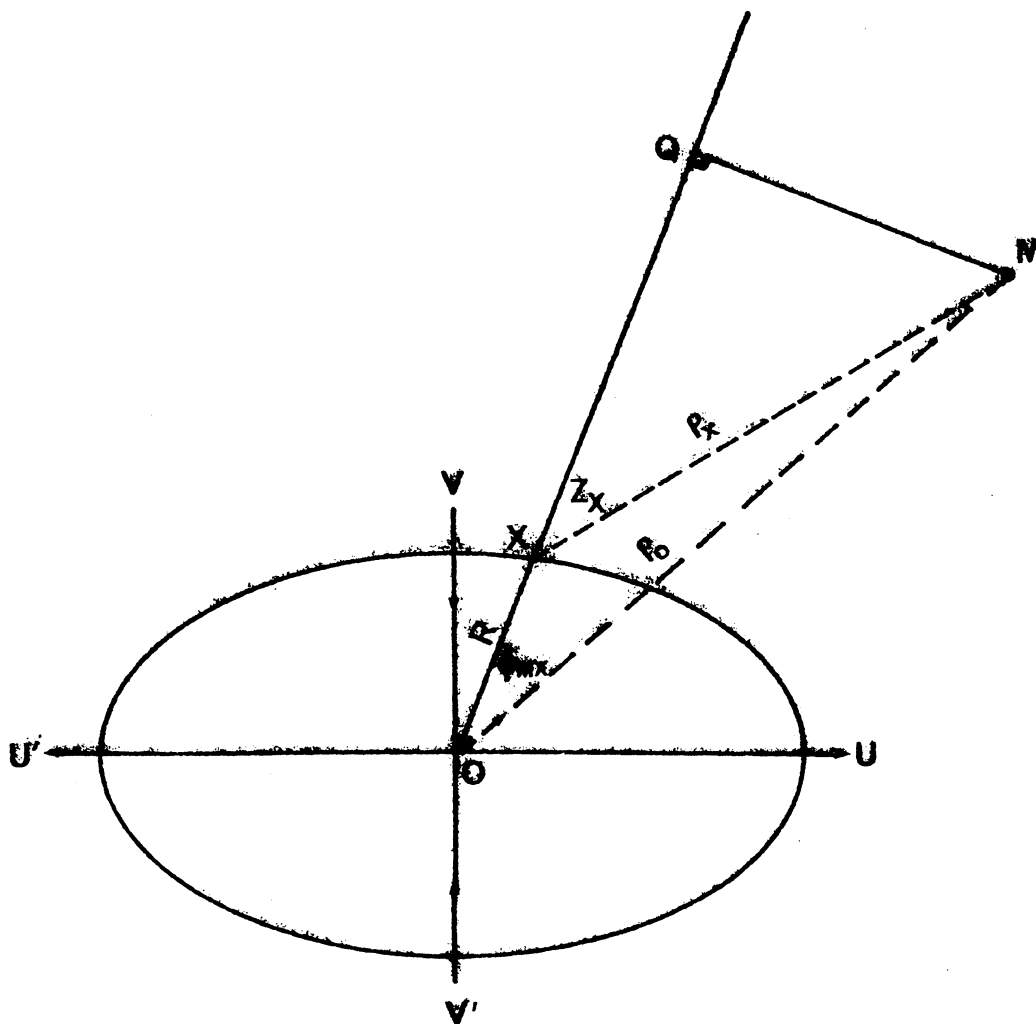


Figure 2-3

Effects of the Gravitational Attraction

Of a Heavenly Body  $M$  on the Earth

between two bodies is a function of the distance between them. This gravitational force at 0 (Figure 2-3) is

$$F_{g0} = KM/\rho_0^2, \quad (2.6)$$

and at X is

$$F_g = KM/\rho_x^2, \quad (2.7)$$

where  $\rho_x$  is the distance between the centre of mass of the moon and point X on the earth's surface. The tide generating force due to the moon M at point X (Figure 2-3) on the earth's surface is defined as the difference between the gravitational force at X and that at the resultant centre of mass of the earth-moon system where the gravitational and centrifugal forces are in equilibrium [Dronkers, 1972].

In terms of potentials, the attracting potential at X and at time t is

$$f_g = KM/\rho_x - KM/\rho_0, \quad (2.8)$$

and the potential of the constant vector field of the centrifugal force is

$$f_c = KM a \cos \phi_{mx} / \rho_0^2, \quad (2.9)$$

where  $\phi_{mx}$  is the zenith distance as shown in Figure 2-3, and a is the mean radius of the earth. From equations 2.8 and 2.9 and making use of the definition of the tide generating force given above, the tide generating potential ( $V_m$ ) due to the moon at X and at time t is [Dronkers, 1972].

$$V_m = KM \left[ \frac{1}{\rho_x} - \frac{1}{\rho_0} - a \cos \phi_{mx} / \rho_0^2 \right] . \quad (2.10)$$

Figure 2-4 shows the distribution on the earth of tide forces of lunar origin. At point A nearest to the moon, the force of attraction is greater than the centrifugal force. The resultant is the tidal force ( $F_t$ ) towards the moon. At C, the centre of the earth, both centrifugal and the gravitational forces are equal. The tidal force at the centre consequently is zero. At B farthest from the moon where the centrifugal force is greater than the attractive force, the tidal force is directed away from the moon.

We can express  $\rho_x$  (equations 2.10) in terms of  $\rho_0$  and  $\phi_{mx}$  using the cosine formula of plane trigonometry given by

$$\rho_x^2 = \rho_0^2 + a^2 - 2a\rho_0 \cos \phi_{mx} . \quad (2.11)$$

Equation 2.11 can be rewritten as

$$\frac{1}{\rho_x} = \frac{1}{\rho_0} \left[ 1 - \frac{2a}{\rho_0} \cos \phi_{mx} - \left( \frac{a}{\rho_0} \right)^2 \right] . \quad (2.12)$$

When  $\frac{1}{\rho_x}$  is expanded in powers of the parallax  $a/\rho_0$  by means of a Taylor series, expansion in zonal harmonics is obtained and equation 2.10 is given as [Godin, 1972]

$$V_m = KM/\rho_0 \left[ P_0(\phi_{mx}) + (a/\rho_0)P_1(\phi_{mx}) + (a/\rho_0)^2 P_2(\phi_{mx}) + (a/\rho_0)^3 P_3(\phi_{mx}) + \dots \right] . \quad (2.12a)$$

The first term of the expansion

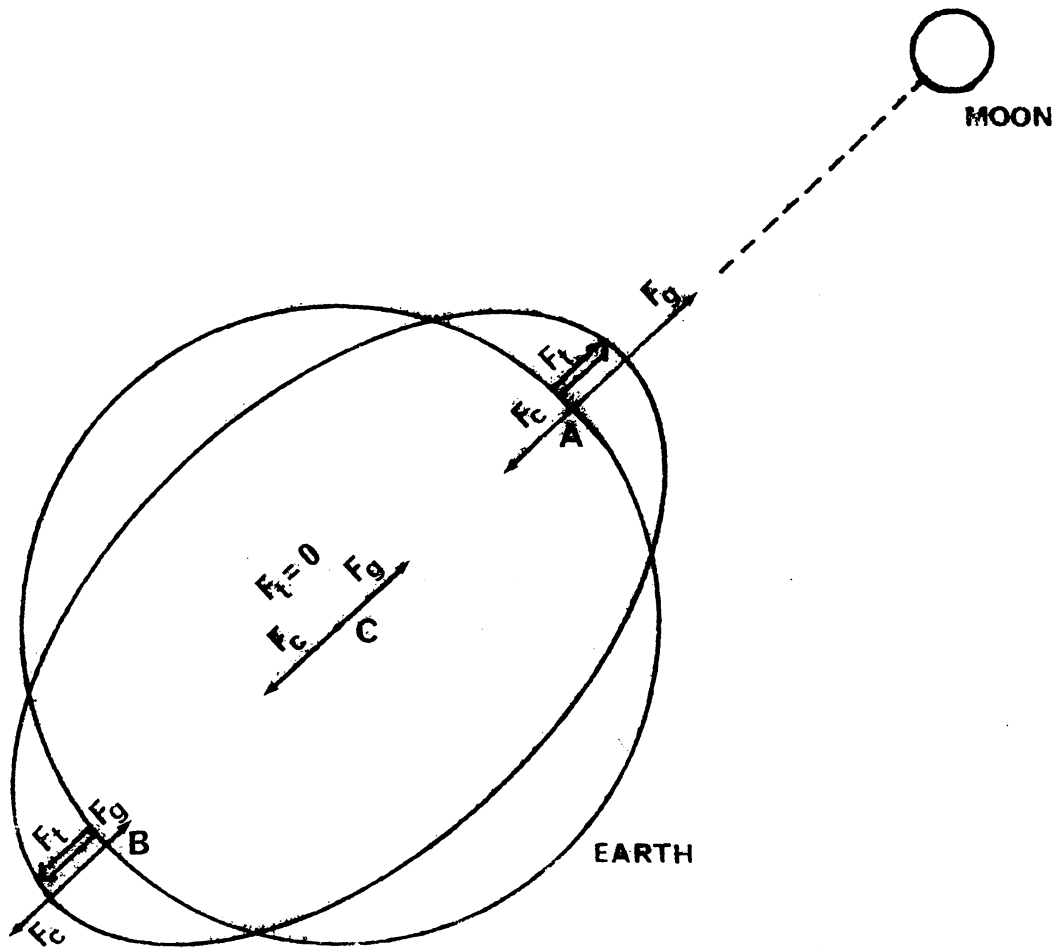


Figure 2-4

Distribution of Tidal Force

$$V_0 = KM/\rho_0 , \quad (2.13a)$$

can be overlooked because it is a constant and hence has no physical significance.

The second term

$$V_1 = KM/\rho_0^2 a \cos \phi_{mx} , \quad (2.13b)$$

is the lunar gravitational force at the centre which is equivalent to the centrifugal force.

The third term is

$$V_2 = KM a^2/\rho_0 \frac{1}{2}(3 \cos^2 \phi_{mx} - 1) . \quad (2.13c)$$

This is the significant term as far as tidal potential is concerned. The fourth term is

$$V_3 = KM a \frac{3}{\rho_0^4} \frac{1}{2}(5 \cos^3 \phi_{mx} - 3 \cos \phi_{mx}) . \quad (2.13d)$$

For practical purposes, the fourth term is of little significance. It must be considered when we are required to determine the potential with a higher degree of accuracy. Henceforth in this report,  $V_2$  is the tidal potential. It is decomposed into constituent frequencies and this, as has been mentioned, is the first step in the harmonic analysis of tidal records.

We can rewrite equation 2.13 as

$$V_m = \frac{3}{2} KM a^2/\rho^3 (\cos^2 \phi_{mx} - \frac{1}{3}) . \quad (2.14)$$

The principal variable in the tide generating potential defined by equation 2.14 is the zenith distance  $\phi_{mx}$ . This quantity changes due to two effects [Dronkers, 1964], namely,

- (i) the daily rotation of the earth about its axis (24 hours) combined with the motion of the moon in its orbit (50 minutes per day) giving a total periodicity of 24 hours, 50 minutes,
- (ii) effects due to moon's motion in its orbit during a lunar month which results in a mean monthly periodicity of its declination  $\delta$  of 27.3 mean solar days.

The other variable in the potential that must be accounted for is  $\rho_0$ , the mean distance of the moon to the earth which varies due to the irregular elliptical nature of lunar orbit.

The expression of the potential as a function of time dependant variables and as a function of position on the earth surface is achieved by transforming our Horizon co-ordinate system to the Hour Angle system using [Smart, 1971]

$$\cos \phi_{mx} = \sin \phi \sin \delta + \cos \delta \cos \phi \cos t, \quad (2.15)$$

where  $\phi$  is the geodetic latitude,  $\delta$  is the declination and  $t$  is the hour angle. We can evaluate  $\cos^2 \phi_{mx}$  in terms of  $\phi$ ,  $\delta$  and  $t$  which after some manipulation yields

$$V_m = G(a, \rho) [\cos^2 \phi \cos^2 \delta \cos 2t + \sin 2\phi \sin 2\delta \cos t + 3(\sin^2 \phi - \frac{1}{3})(\sin^2 \delta - \frac{1}{3})] , \quad (2.16)$$

in which  $G(a, \rho)$  is defined as the Doodson constant, namely  $G(a, \rho) = \frac{3}{4} \text{KM} \frac{a^2}{c^3}$  ( $c$  is the mean semi-axis of the orbital ellipse of the moon).

Equation 2.16 contains the variables  $\rho$ ,  $\delta$ ,  $t$  which are dependant on time. The first term of the equation containing  $\cos 2t$  includes the semi-diurnal constituents with periods approximating half a lunar day. The second term containing  $\cos t$  determines the diurnal constituents with periods approximating a lunar day. The third term is independent of  $t$  and hence contains the long period constituents. It is only subject to variations in declination  $\delta$  and distance  $\rho$  of the celestial body. We have now been able to decompose the tidal potential into 3 frequency bands

- 0 - for long period constituents,
- 1 - for diurnal constituents,
- 2 - for semi-diurnal constituents.

This is only a step towards the complete decomposition of the tidal potential into the numerous periodic constituents. For the complete decomposition, the work of Darwin and Doodson are important. Darwin's decomposition provides readily the most important constituents and their relative importance while Doodson's method is more suitable for rigorous developments and provides a greater number of



constituents.

### 2.1.3 Development According to Darwin

This development is based on deriving relations for  $\sin \delta$  and  $\cos \delta \cos t$ , which occur in equation 2.16 in terms of

$t$  - the local solar time,

$s$  - the longitude of the moon referred to the equator,

$h$  - the mean ecliptic longitude of the sun.

Darwin used the old lunar theory and all quantities were given with respect to the moon's orbit projected onto the celestial equator. He considered

$P$  - the ecliptic longitude of the moon's perigee,

$n$  - the ecliptic longitude of the moon's nodes,

$P_s$  - the ecliptic longitude of the sun's perigee,

as constant over one year.

Referring to Figure 2-5, the relations are derived from right spherical triangles  $MAM'$  and  $MX'M'$  and the oblique triangle  $MAX'$ .  $A$  is a point of intersection of the lunar orbit and the equator,  $X'$  and  $M'$  are the projections of  $X$  and  $M$  onto the equator [Dronkers, 1964 Page 59]. From triangle  $MAM'$  and  $MX'M'$ , the sine rule of spherical trigonometry yields

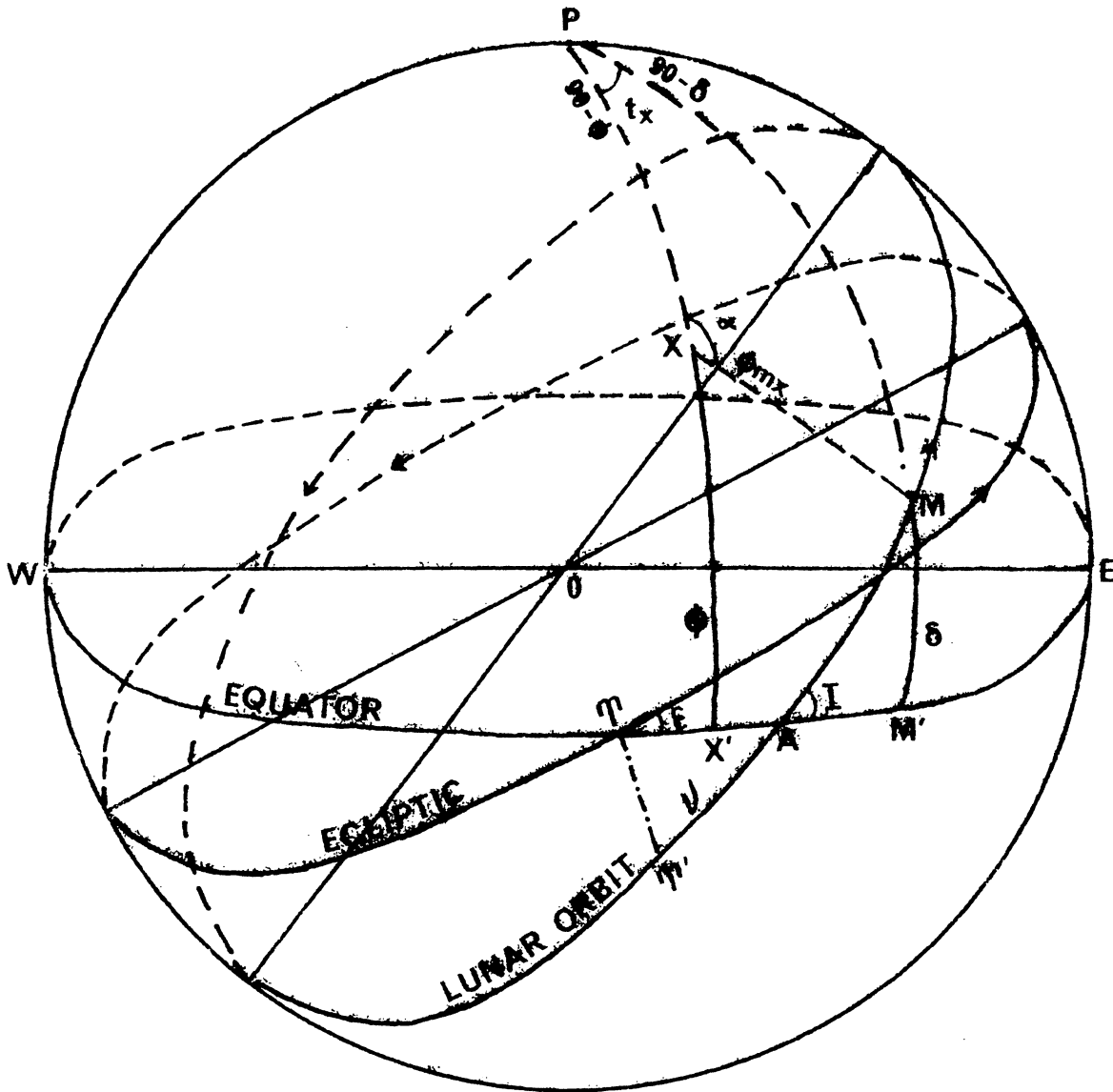


Figure 2-5

**Orbital Parameters**

$$\sin \delta = \sin I \sin(s - v + k). \quad (2.17)$$

$$\cos \delta \cos t - \cos \chi, \quad (2.18)$$

where  $I$  is the angle between the orbit of the moon and the celestial equator,  $s$  is the longitude of the mean moon on the equator,  $v$  is the distance between the referred equinox  $\gamma'$  and the intersection of the lunar orbit with the equator at  $A$ ,  $\chi$  is the arc  $MX'$  and arc  $AM = s - v + k$ .  $k$  is the difference between the true longitude of the moon ( $s'$ ) measured from  $\gamma'$  ( $\gamma'M$ ) and the longitude of the mean moon in the equator  $s$ . From oblique triangle  $MAX'$  and using the cosine formula we have that

$$\begin{aligned} \cos \chi = & \cos(15^\circ tx + h - v) \cos(s - v + k) \\ & + \sin(15^\circ tx + h - v) \sin(s - v + k) \cos I \end{aligned} \quad (2.19)$$

in which  $h$  is the mean ecliptic longitude of the sun and  $v$  is the right ascension of  $A$ ,  $15^\circ$  of arc is equal to one hour in time. The terms  $\sin^2 \delta$ ,  $\sin 2\delta \cos t$  and  $\cos^2 \delta \cos 2t$  which are contained in the potential formula (equation 2.16), can be determined from equations 2.17, 2.18 and 2.19 in terms of the orbital elements  $tx$ ,  $s$ ,  $h$  and  $v$ . When these are substituted back into equation 2.16, we obtain a series of harmonic terms of which the arguments depend on the rotation of the earth ( $15^\circ tx$ ), the mean motion of the moon in its orbit ( $s$ ) and the mean motion of the earth in orbit ( $h$ ) namely.

$$\begin{aligned}
V_m = G(a, \rho) \{ & \cos^2 \phi \left[ \cos^4 \frac{I}{2} \cos(30^\circ tx - 2s - 2h - 2v - 2v - 2k) \right. \\
& + \frac{1}{2} \sin^2 I \cos(30^\circ tx + 2h - 2v) \\
& + \sin^4 \frac{I}{2} \cos(30^\circ tx + 2s + 2h - 2v - 2v + 2k) \left. \right] \\
& + \sin 2\phi \left[ \sin I \cos^2 \frac{I}{2} \cos(15^\circ tx - 2s + h + 2v - v \right. \\
& \quad \left. - 2k - 90^\circ) + \frac{1}{2} \sin 2I \cos(15^\circ tx + h - v + 90^\circ) \right. \\
& + \sin I \sin^2 \frac{I}{2} \cos(15^\circ tx + 2s + h - 2v - v - 2k + 90^\circ) \left. \right] \\
& + (1 - 3 \sin^2 \phi) \left[ \frac{2}{3} - \sin^2 I + \sin^2 I \cos(s - v + k) \right] \}.
\end{aligned} \tag{2.20}$$

In the development for solar constituents, the terms  $v$  and  $v$  will vanish and angle  $I$  will change to  $\epsilon$ .

#### 2.1.4 Development According to Doodson

Doodson's method principally involves the use of a rigorous expansion of the ecliptic longitude and latitude of the moon. For the development of  $\sin \delta$  and  $\cos \delta \cos t$ , he introduced the ecliptic longitude  $\lambda_m$  and latitude  $\beta_m$  of the moon and the local siderreal time  $\theta$  of the point X (Figure 2-3) on the earth's surface. The equations are

$$\sin \delta = \sin \epsilon \sin \lambda_m \cos \beta_m + \cos t \sin \beta_m, \tag{2.21}$$

$$\begin{aligned}
\cos \delta \cos t = & \cos \beta_m \cos \lambda_m \cos \theta + (\cos \epsilon \cos \beta_m \sin \lambda_m - \\
& - \sin \epsilon \sin \beta_m) \sin \theta, \tag{2.22}
\end{aligned}$$

where  $\epsilon$  is the obliquity of the ecliptic.

Finally the potential  $V_m$  is developed as the sum of periodic functions of six variables, namely,  $tx$ ,  $s$ ,  $h$ ,  $P$ ,  $n$  and  $Ps$ .

Doodson obtained 400 periodic constituents from his development of which the principal ones are listed in Table 2-1 [Vaniček, 1973].

The constituent frequencies can be described in mathematical terms using Doodson numbers and the astronomical variables, namely

$$\omega_k = \bar{k}\bar{f} = k_1 f_1 + k_2 f_2 + k_3 f_3 + k_4 f_4 + k_5 f_5 + k_6 f_6, \quad (2.23)$$

$$(k_X = 0 \pm 1 \pm 2).$$

$\bar{f}$  is a six dimensional vector whose components are the basic frequencies of the motions of the earth, the moon and the sun, namely

$f_1^{-1}$  is the period of the earth's rotation  $\tau_x$  (1 day),

$f_2^{-1}$  is the period of moon's orbital motion  $\delta$  (1 month),

$f_3^{-1}$  is the period of earth's orbital motion  $\dot{h}$  (1 year),

$f_4^{-1}$  is the period of lunar perigee  $\dot{P}$  (8.85 years),

$f_5^{-1}$  is the period of regression of lunar nodes  $\dot{N}$  (18.61 years),

$f_6^{-1}$  is the period of solar perigee  $\dot{P}_s$  (21000 years).

$f_6$  is usually omitted because it is insignificant.  $k_X = 0, 1, 2$  refers to the tidal species, 0 for long period, 1 for diurnal and 2 for semi-diurnal.  $(k_1, k_2)$  is called the group number.  $(k_1, k_2, k_3)$  is called the constituent number.

With the constituent frequencies determined, which are the same anywhere on the earth's surface, the first step in the harmonic analysis is now completed. In the next

| Symbol                  | Velocity<br>per hour | Amplitude $10^5$ | Origin<br>(L, lunar; S, solar) |
|-------------------------|----------------------|------------------|--------------------------------|
| Long period components  |                      |                  |                                |
| $M_o$                   | 0°,000000            | + 50458          | L constant flattening          |
| $S_o$                   | 0°,000000            | + 23411          | S constant flattening          |
| $S_a$                   | 0°,041067            | + 1176           | S elliptic wave                |
| $S_{sa}$                | 0°,082137            | + 7287           | S declinational wave           |
| $M_m$                   | 0°,544375            | + 8254           | L elliptic wave                |
| $M_f$                   | 1°,098033            | + 15642          | L declinational wave           |
| Diurnal components      |                      |                  |                                |
| $Q_1$                   | 13°,398661           | + 7216           | L elliptic wave of $O_1$       |
| $O_1$                   | 13°,943036           | + 37689          | L principal lunar wave         |
| $M_1$                   | 14°,496694           | - 2964           | L elliptic wave of $^m K_1$    |
| $\pi_1$                 | 14°,917865           | + 1029           | S elliptic wave of $P_1$       |
| $P_1$                   | 14°,958931           | + 17554          | S solar principal wave         |
| $S_1$                   | 15°,000002           | - 423            | S elliptic wave of $^s K_1$    |
| $^m K_1$                | 15°,041069           | - 36233          | L declinational wave           |
| $^s K_1$                | 15°,041069           | - 16817          | S declinational wave           |
| $\psi_1$                | 15°,082135           | - 423            | S elliptic wave of $^s K_1$    |
| $\phi_1$                | 15°,123206           | - 756            | S declinational wave           |
| $J_1$                   | 15°,585443           | - 2964           | L elliptic wave of $^m K_1$    |
| $OO_1$                  | 16°,139102           | - 1623           | L declinational wave           |
| Semi-diurnal components |                      |                  |                                |
| $2N_2$                  | 27°,895355           | + 2301           | L elliptic wave of $M_2$       |

Table 2-1 Principal Tidal Constituents As Derived by Doodson.

Table 2-1 -continued .

| Symbol                | Velocity<br>per hour | Amplitude $10^5$ | Origin<br>(L, lunar; S, solar) |
|-----------------------|----------------------|------------------|--------------------------------|
| $\mu_2$               | 27°,968208           | + 2777           | L variation wave               |
| $N_2$                 | 28°,439730           | + 17387          | L major elliptic wave of $M_2$ |
| $\nu_2$               | 28°,512583           | + 3303           | L evection wave                |
| $M_2$                 | 28°,984104           | + 90812          | L principal wave               |
| $\lambda_2$           | 29°,455625           | - 670            | L evection wave                |
| $L_2$                 | 29°,528479           | - 2567           | L minor elliptic wave of $M_2$ |
| $T_2$                 | 29°,958933           | + 2479           | S major elliptic wave of $S_2$ |
| $S_2$                 | 30°,000000           | + 42286          | S principal wave               |
| $R_2$                 | 30°,041067           | - 354            | S minor elliptic wave of $S_2$ |
| $m_{K_2}$             | 30°,082137           | + 7858           | L declinational wave           |
| $s_{K_2}$             |                      |                  |                                |
| Ter-diurnal component |                      |                  |                                |
| $M_3$                 | 43°,476156           | - 1188           | L principal wave               |

section, the least squares harmonic analysis of observed tidal records, to determine the tidal constants  $H_k$  and  $g_k$ , where  $H_k$  is the amplitude of the constituent  $k$  and  $g_k$  the phase lag of the constituent  $k$  at the observed station, is described.

## 2.2 Least Squares Harmonic Analysis and Prediction of Tides

The height of tide  $h(t)$  at any place and at any time  $t$  can be expressed as the sum of harmonic terms [Dronkers, 1972]

$$h(t) = s_0 + \sum_{k=1}^{\infty} H_k \cos(\omega_k t + \alpha_k), \quad (2.24)$$

where  $s_0$  is the height of mean water level above the datum in use,  $\omega_k$  is the constituent frequency,  $H_k$  is the amplitude of the constituent  $k$  and  $\alpha_k$  is the initial phase of the constituent. The number of constituents included will depend on the accuracy required for prediction. For ordinary hydrographic works, the constituents  $M_2$ ,  $S_2$ ,  $N_2$ ,  $O_1$ ,  $K_1$ ,  $P_1$  are sufficient to yield an accuracy of 0.2 m in a prediction.  $\alpha_k$  depends on the varying mean longitudes of the moon's perigee and sun's perigee with periods of approximately 8.61 and 21000 years respectively and the ecliptic longitude of the moon's ascending node with a period of 18.61 tropical years. To take these effects into account,  $f_5$  and  $f_6$  constituents are eliminated and a node factor  $f_k$  and a correction for equilibrium argument  $U_k$  are introduced.

Equation 2.24 is rewritten as

$$h(t) = s_0 + \sum_{k=1}^N f_k H_k \cos(\omega_k t + (V_k + U_k) - X_k), \quad (2.25)$$



in which  $(V_k + U_k)$  is the value of the equilibrium argument of the constituent  $k$  when  $t = 0$ , generally called the astronomical argument,  $X_k$  is the phase lag of the tidal constituent behind the phase of the corresponding equilibrium constituent at Greenwich,  $N$  is the number of constituents in use.

All tide observations are made on local standard time, often referred to as zone time and denoted as ZT. Equation 2.25 therefore has to be modified so that allowance is made both for the zone time and the local longitude since the meridian of the observing station and the meridian defining zone time are usually not coincident (Figure 2-6).

If  $(V_k - U_k)$  is the phase of the equilibrium constituent  $k$  at the Greenwich,  $P (= 0, 1, 2)$  is the tide species number, 0 for long period, 1 for diurnal and 2 for semi-diurnal and  $\lambda_x$  is the geodetic longitude of the point, say  $X_2$  (Figure 2-6) west of the Greenwich, then  $(V_k + U_k) - P\lambda_x$  is the phase expressed in Greenwich mean time of the equilibrium constituent  $k$  of the tide species  $P$  at the point  $X_2$  west of Greenwich. This is now transformed into the zone time of the place. If the correction for zone time is  $\Delta T$  (where  $\Delta T$  is negative west of Greenwich and positive east of Greenwich) and the frequency of the constituent is  $\omega_k$ , we must subtract  $\omega_k \cdot \Delta T$  from the phase of the equilibrium tide. Thus with respect to the point  $X_2$  west of Greenwich,  $V_k + U_k - P\lambda + \omega_k \cdot \Delta T$  is the phase of the equilibrium tide expressed in the local zone time.

If  $g_k$  is the phase lag  $X_k$  corrected for longitude and

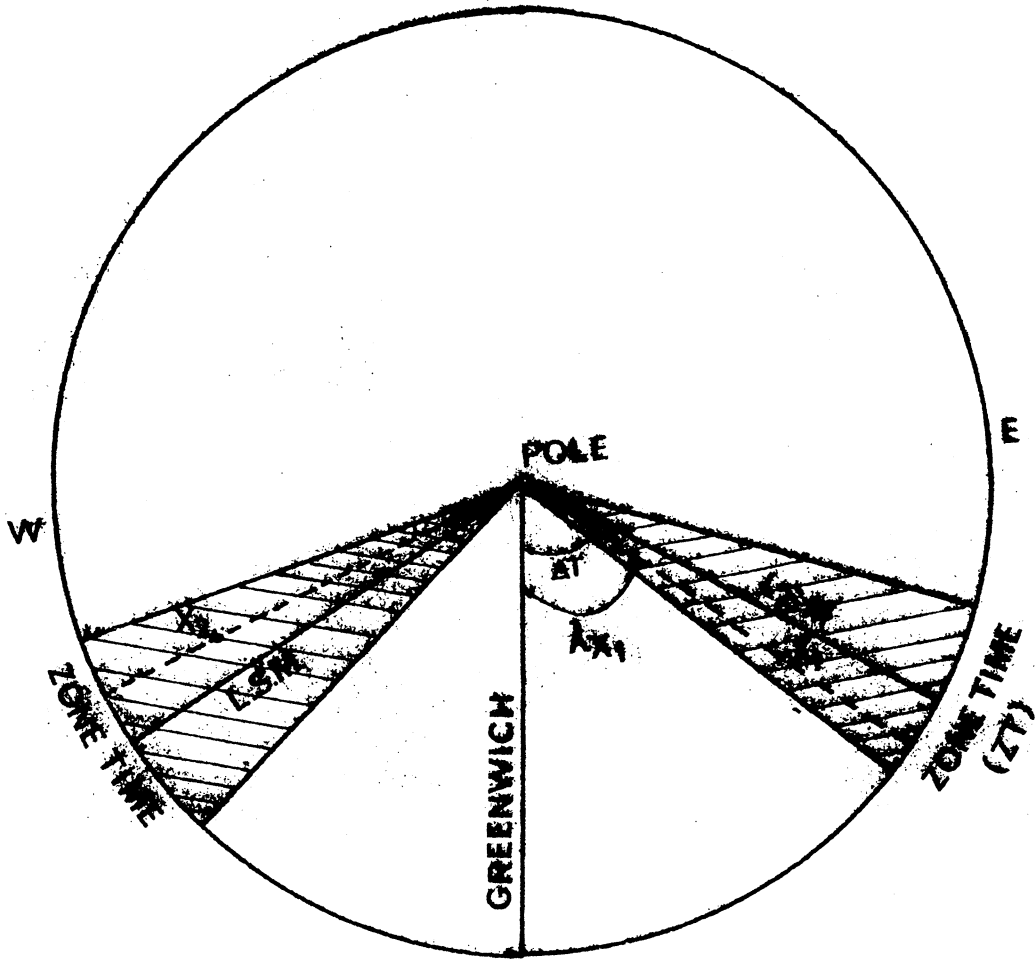


Figure 2-6  
Time Relationships

zone time, then we have that

$$V_k + U_k - g_k = V_k + U_k - P\lambda - \omega_k \cdot \Delta T - X_k,$$

$$g_k = X_k + P\lambda + \omega_k \cdot \Delta T. \quad (2.26)$$

The determination of  $H_k$  in equation 2.25 and  $g_k$  in equation 2.26 are the objectives in the harmonic analysis of tides. They are determined from a series of observed tides at a tide gauge station and are called the harmonic constants for that station. The estimation of these constants for a station is improved when more observations are available.

From equation 2.25, using trigonometric relations for compound angles

$$f_k H_k \cos[\omega_k \cdot t + (V_k + U_k) - X_k] \equiv f_k H_k \cos((V_k + U_k) - X_k) \cos(\omega_k \cdot t) + (f_k H_k \sin((V_k + U_k) - X_k) \sin(\omega_k \cdot t)). \quad (2.27)$$

If we let

$$f_k H_k \cos((V_k + U_k) - X_k) = A_k, \quad (2.28)$$

$$f_k H_k \sin((V_k + U_k) - X_k) = B_k, \quad (2.29)$$

equation 2.25 is rewritten as

$$h(t) = S_0 + \sum_{k=1}^N A_k \cos(\omega_k \cdot t) + \sum_{k=1}^N B_k \sin(\omega_k \cdot t). \quad (2.30)$$

Equation 2.30 is a trigonometric polynomial that can predict the observed time series  $h(t)$  at time  $t$  in the given interval of time. Least squares approximation methodology [Vanicek and Wells, 1972; Moritz, 1977; Appendix I] can be used to determine the coefficients  $S_0$ ,  $A_k$ ,  $B_k$

( $k = 1, 2, 3, \dots, N$ ). The number of coefficients to be solved is

$$U = 2N + 1, \quad (2.31)$$

where  $N$  is the number of constituent frequencies used.

We can choose our base functions as

$$\psi \equiv \{1, \cos \omega_1 t, \sin \omega_1 t, \dots, \cos \omega_N t, \sin \omega_N t\}. \quad (2.32)$$

The Vandermonde's design matrix  $A$  is

$$A_{M \times U} = \begin{bmatrix} 1, \cos \omega_1 t_1, \sin \omega_1 t_1, \dots, \cos \omega_N t_1, \sin \omega_N t_1 \\ 1, \cos \omega_1 t_2, \sin \omega_1 t_2, \dots, \cos \omega_N t_2, \sin \omega_N t_2 \\ \vdots \\ 1, \cos \omega_1 t_m, \sin \omega_1 t_m, \dots, \cos \omega_N t_m, \sin \omega_N t_m \end{bmatrix}, \quad (2.33)$$

in which  $m$  equals the number of measurements  $h(t)$  that have been made. For weights, we can consider each observation as having been made independently with equal amount of reliability. The error in observations ( $\sigma_{X_L}$ ), can be taken to be equal to the resolution of the tide gauge used so that

$$\sum_{L} = \text{diag} \left\{ \sigma_{L_1}^2, \sigma_{L_2}^2, \dots, \sigma_{L_m}^2 \right\},$$

and the corresponding weight matrix is

$$P_{m \times m} = \sum_L^{-1} = \text{diag} \left\{ \frac{1}{\sigma_{L_1}^2}, \frac{1}{\sigma_{L_2}^2}, \dots, \frac{1}{\sigma_{L_m}^2} \right\}, \quad (2.34)$$

in which  $\sigma_0^2$  (the a priori variance factor) is taken as unity.

The solution for the vector of coefficients is given

as

$$\hat{C} = (A^T P A)^{-1} A^T P F, \quad (2.35)$$

in which  $\hat{C} = [S_0, A_1, B_1, A_2, B_2 \dots A_k, B_k]^T$  .

The solution for the residual vector is

$$\hat{V} = A\hat{C} - F \quad , \quad (2.36)$$

where  $F$  is a vector of observed heights.

The associated variance covariance matrix of the vector of coefficients is

$$\Sigma_{\hat{C}} = \hat{\sigma}_0^2 [A^T P A]^{-1} \quad , \quad (2.37)$$

where  $\hat{\sigma}_0^2$  is the estimated variance factor given by

$$\hat{\sigma}_0^2 = \frac{\hat{V}^T P \hat{V}}{df} \quad , \quad (2.38)$$

$df$  represents the degree of freedom given in this case by the number of observations minus the number of coefficients ( $df = m - u$ ).

With the coefficients  $S_0, A_k, B_k$  determined, equations 2.26, 2.28 and 2.29 yield the harmonic constants  $H_k$  and  $g_k$ . Note that if however it is not intended to predict the tides in the past or in the future, the constants need not be computed. The tide at any time  $t$  in the time interval can be predicted using the polynomial.

From 2.28 and 2.29

$$\frac{f_k H_k \sin((V_k - U_k) - X_k)}{f_k H_k \cos((V_k + U_k) - X_k)} = \tan((V_k + U_k) - X_k) = \frac{B_k}{A_k} \quad ,$$

or

$$((V_k + U_k) - X_k) = \tan^{-1}(B_k/A_k) \quad , \quad (2.39)$$

and

$$f_k H_k \cos((V_k + U_k) - X_k) = A_k \quad ,$$

$$H_k = A_k/f_k \cos((V_k + U_k) - X_k) , \quad (2.40)$$

or

$$f_k H_k \sin((V_k + U_k) - X_k) = B_k ,$$

$$H_k = B_k/f_k \sin((V_k + U_k) - X_k) . \quad (2.40a)$$

To completely solve our problem, we have to determine the astronomical argument  $(V_k + U_k)$  and the nodal factor  $(f_k)$ . The values are usually tabulated in tide tables (eg. Admiralty Tide Tables), or they can be computed.

The astronomical argument is given as [Godin, 1972; pp. 171-178]

$$V_k(t) = k_1 \hat{\tau} + k_2 \hat{S} + k_3 \hat{h} + k_4 \hat{P} + k_5 \hat{N} + k_6 \hat{Ps} . \quad (2.41)$$

where  $\hat{\tau}$ ,  $\hat{S}$ ,  $\hat{h}$ ,  $\hat{P}$ ,  $\hat{N}$  and  $\hat{Ps}$  are the values of the astronomical variables at the instant of time  $t$  from the origin of time and are given as

$$\begin{aligned} \hat{S} &= S_0 + \Delta t \dot{S} , \\ \hat{h} &= h_0 + \Delta t \dot{h} , \\ \hat{P} &= P_0 + \Delta t \dot{P} , \\ \hat{N} &= N_0 + \Delta t \dot{N} , \\ \hat{Ps} &= Ps_0 + \Delta t \dot{Ps} , \\ \hat{\tau} &= 0.0416 (\text{hh mm}) + \hat{h} - \hat{S} . \end{aligned}$$

$S_0$ ,  $h_0$ ,  $P_0$ ,  $N_0$  and  $Ps_0$  are the values of the astronomical variables at the time  $t = 0$ , hh mm represents the hours and minutes of the day,  $\dot{S}$ ,  $\dot{h}$ ,  $\dot{P}$ ,  $\dot{N}$ ,  $\dot{Ps}$  are the rates of change of the astronomical variables in cycles per mean lunar day.  $U_k$  is the phase of the astronomical argument  $(V_k)$  at

time  $t = 0$ .

The nodal (modulation) factor is given by [Godin, 1972]

$$f_k = 1 + \sum_{j=1}^n |r_{kj}| \exp[2\pi i(\Delta k_4(j)\hat{P} + \Delta k_5(j)\hat{N} + k_6(j)\hat{P}_S)],$$

(2.42)

in which  $r_{kj}$  is a complex number which depends on  $\Delta k_4$ ,  $\Delta k_5$  and  $\Delta k_6$ . The  $j$ 's inside the differences in Doodson numbers indicate that they depend on a specific constituent within a cluster.

It is important to note that in the discussion so far, there was no mention of removing the noise part of the observed series before the analysis is made. The harmonic constants obtained are therefore likely to include other effects beside those of the astronomic forces and are consequently in a certain measure variable. The harmonic analysis should be based on a series of very selective filterings so as to permit isolation of an oscillation having a maximum tide/noise ratio. Godin [1972] has given several filters that could be used to eliminate the noise part or suppress certain frequencies.

Vaniček [1970] pointed out that there is an obvious danger in removing the noise part of a series when the magnitudes are not known. On the other hand, it is usually equally detrimental to leave these constituents unattended because they may distort the spectral image of the series to a considerable degree. He described a method of least squares spectral analysis that could be used to analyse a time series and locate the frequencies accurately

without first removing the noise part.

Mosetti and Manca [1972] described a number of methods for separating a certain number of tidal constituents by means of successive approximations and thus to completely extract astronomic tide from the tidal records. The frequency interval in which the tidal constituents occur are divided into a number of wave groups, the periods within each group being very close to each other but sufficiently distinct from the periods of constituents in all other groups. By drawing the graph of oscillations in each group, it is easy to see that the modulations are perturbed to some extent due to interference phenomena from waves within the group. If we are dealing with series extending over a fairly long period, it is possible to evaluate the intervals on the record that are least perturbed and where the amplitudes vary with regularity dictated by astronomic laws. The harmonic constants can then be computed for those intervals.

## 2.3 Tidal Analysis and Prediction by Response Method

### 2.3.1 General

Munk and Cartwright [1966] presented an entirely different method of tidal analysis and prediction. They applied the theory of time series to the tidal observations at a gauge station to determine certain coefficients which replaced the amplitudes  $H_k$  and the phase lags  $g_k$  of the tidal constituents as in the harmonic analysis. Even



though the theory of this method is more involved than the harmonic method, the authors claim that the response method gives a simpler and physically more meaningful representation of tides than the harmonic method. Unlike the traditional harmonic method which attempts to express the tides as the sum of harmonic functions of time, the response method expresses tide as the weighted sum of the past, present and future values of a relatively small number of time varying input functions.

Dronkers [1972] described the method as a more empirical modification of the equilibrium tide based on the theory of time series. He added that the principal advantage of the response method is that the total number of coefficients is less than the number of constituents used for the harmonic prediction of comparable accuracy. In the response method we deal with complete potential instead of a set of discrete frequencies as in the harmonic method.

Lambert [1974] noted that the principal advantage of response method over the harmonic method lies in the fact that separate admittance functions (Fourier transform of response weights) can be calculated for sufficiently distinct uncorrelated inputs, thus making the method adaptable for earth tide analysis.

The response method of tide analysis and prediction as developed by Munk and Cartwright [1966] is applied to various observed series to obtain frequency dependent

admittances that describe the tidal characteristics in a similar sense to what can be deduced from the traditional harmonic constants. To bridge the gap between the response and harmonic methods, Zetler, Cartwright and Munk [1969] have described procedures for deriving harmonic constants from the response admittances. They showed that the harmonic constants  $H_k$  and  $g_k$  of a tide constituent  $k$  can be determined for a place using response analysis and the result is compatible with the conventional harmonic analysis.

### 2.3.2 Brief Outline of the Theory of Response Method

The tidal potential can be generated as a time series  $V(t)$  and an attempt can be made at predicting the height of tide for a time  $t$  as the weighted sum of the past and present values of the potential,

$$\tilde{h}(t) = \int W(s) V(t - \tau s). \quad (2.43)$$

The weights  $W(s)$  are determined such that the prediction error  $h(t) - \tilde{h}(t)$  is a minimum in the least squares sense,  $\tau s$  is the time lag used in the argument of the potential.

The weights represent the sea level response at the place of interest to a unit impulse

$$V(t) = \delta(t).$$

In the response approach of Munk and Cartwright,  $V(t)$  is expressed in spherical harmonics as

$$V(\phi, \lambda, t) = g \sum_{n=0}^{\infty} \sum_{m=0}^n [a_n^m(t) U_n^m(\phi, \lambda) + i b_n^m(t) V_n^m(\phi, \lambda)]. \quad (2.44)$$

Here  $U_n^m + i V_n^m$  are a set of complex spherical harmonics of order  $m$  and degree  $n$ ,  $a(t)$ ,  $b(t)$  are the amplitudes of the real and imaginary parts of the spherical harmonics and can be computed for any desired time interval for any location.

The prediction formalism becomes [Munk and Cartwright, 1966]

$$\tilde{h}(t) = \sum_{mn} \sum_s [U_n^m(s) a_n^m(t - \tau s) + i V_n^m(s) b_n^m(t - \tau s)]. \quad (2.45)$$

Letting

$$W_n^m(s) = U_n^m(s) + i V_n^m(s) .$$

and

$$C_n^m(t - s) = a_n^m(t - \tau s) - i b_n^m(t - \tau s),$$

equation 2.45 is rewritten as

$$\tilde{h}(t) = \sum_{mn} \sum_s W_n^m(s) C_n^m(t - \tau s). \quad (2.46)$$

The weights  $W_n^m(s)$  define the relation between the linear part of the tide and the equilibrium tide, thus the determination of  $W_n^m(s)$  is the essential point in the response method.

### III COTIDAL CHARTS AND THEIR USES

#### 3.0 Introduction

In Chapter II we have seen how the tidal constituent frequencies are obtained from the decomposition of tidal potentials and how the tidal characteristic for a location, that is, the tidal constants (amplitude  $H_k$  and phase lag  $g_k$  for any constituent  $k$ ) for major constituents can be determined using the harmonic or response methods of tidal analysis. In this chapter, the types and methods of constructing cotidal charts and their uses, are discussed.

#### 3.1 Types of Cotidal Charts

##### 3.1.1 Range/Time Cotidal Charts

Most often, a range/time cotidal chart is constructed by graphical means. On it, two sets of curves connect points having equal range differences (or range ratios) and points having simultaneous high and low waters [Admiralty Manual of Hydrographic Surveying, 1969]. All cotidal curves indicate a relationship to the tides at the reference gauge station. Figure 3-1 illustrates a typical range/time cotidal chart. The range curves (shown by pecked lines) indicate the range ratios of the tide at the reference station A. At B for example, the tidal range is 0.65 times the range at A. The time curves (shown by full lines) indicate time lags or corrections which must be applied to the times of high or low waters at the reference gauge station to obtain the times of high or

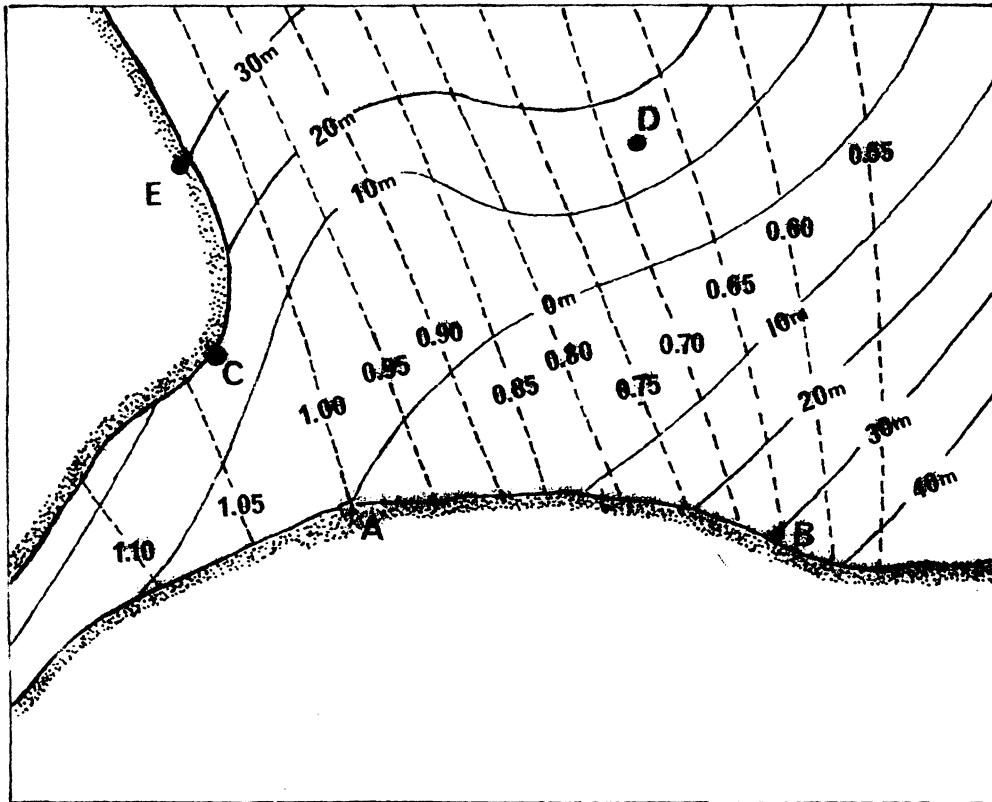


Figure 3-1

Range/Time Co-Tidal  
Chart

low waters at a place of interest.

To construct this type of cotidal chart, simultaneous tide observations are made at the reference station and at other well distributed temporary tide stations such as at points B, C, D and E in Figure 3-1. From mean high waters and mean low waters, the mean range is obtained for each station. The range ratios are determined from the relation: mean range at a gauge station/mean range at the reference station. The mean time lag for each station is determined by finding the mean time differences between the occurrence of high and low waters at the reference station and at other gauge stations. Both sets of cotidal curves are interpolated in between stations as contours are interpolated in between spot heights for a topographic map.

### 3.1.2 Amplitude/Phase Cotidal Charts

This type of cotidal chart is referred to as being semi-graphical. It is more difficult to produce and more complicated to use than a range/time cotidal chart but, could be more reliable and more versatile. The number of such charts needed for an area would be equal to the number of constituent frequencies being taken into account for our tidal predictions. For ordinary practical purposes in hydrographic surveying, four major constituents are considered, namely  $M_2$ ,  $S_2$ ,  $K_1$  and  $O_1$  [Admiralty Manual of Hydrographic Surveying, 1969]. This means that four cotidal charts would be needed each containing two sets of

curves. Figure 3-2 illustrates one such cotidal chart of an area for the  $M_2$  constituent. The full lines connect points having equal values of phase lag  $g_m$  in degrees and the pecked lines connect points having equal amplitudes  $H_m$ .

To produce the amplitude/phase cotidal charts, tide gauges are set up at well distributed locations in the area such that tidal characteristics should as much as possible vary linearly from one gauge station to another. This means that there should be no major physical features or structures which may influence the propagation of tidal waves between any two tide stations. (For example, Larsen [1977] in his study of the tides in the Pacific Ocean near the Hawaiian Islands, observed that the phase lag of the  $M_2$  semi-diurnal tide differs by  $46^\circ$  between the nearby tide stations at Mokuoloe and Honolulu that are on the opposite sides of the Hawaiian ridge but differs by only  $15^\circ$  between Mokuoloe and a distant station at Hilo that are on the same side of the ridge. Also for the  $K_1$  diurnal tide, the differences are found to be  $8^\circ$  and  $3^\circ$  respectively). Tides are observed at the stations for a minimum period of 29 days. The tidal records are then analysed using the harmonic or the response method to determine the harmonic constants  $H_k$  and  $g_k$  for each constituent frequency at each gauge station. The amplitude and phase lag curves are then interpolated as contours are interpolated for a topographic map.

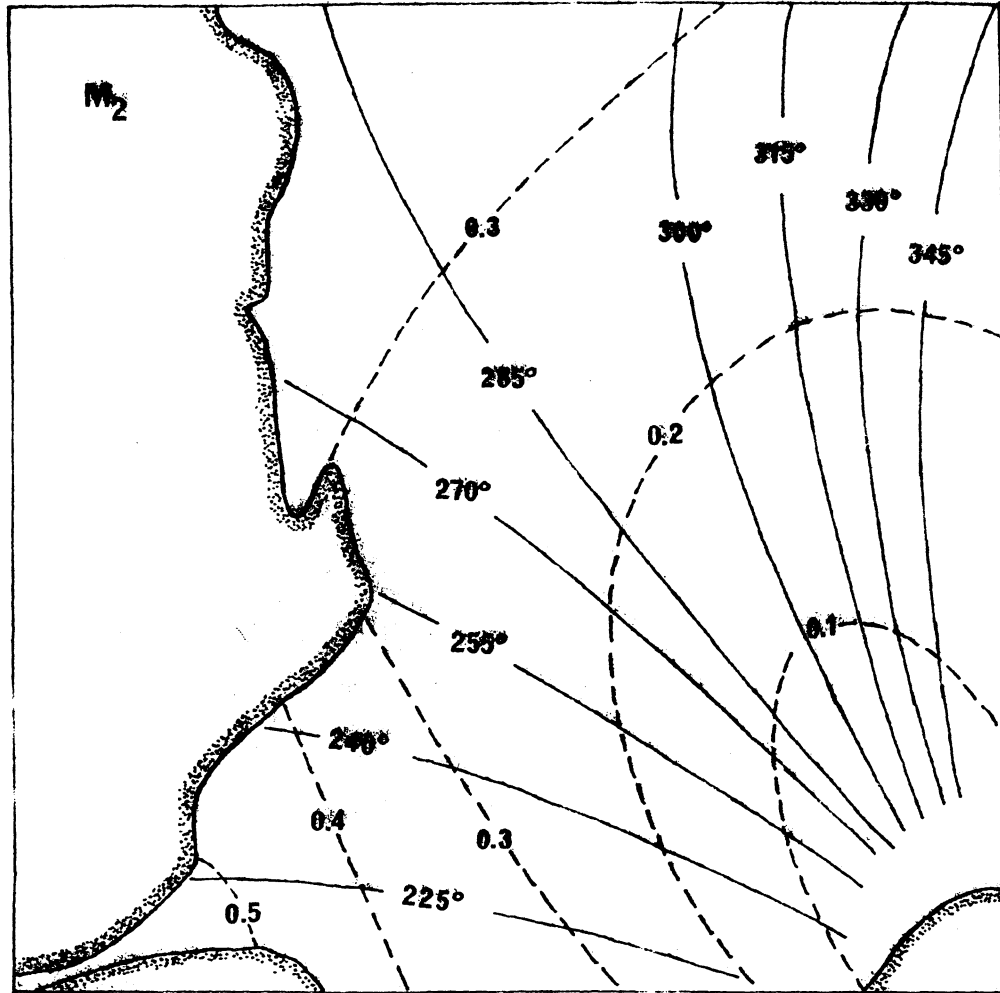


Figure 3-2

Amplitude/Phase Co-Tidal Chart for M<sub>2</sub>



The amplitude/phase cotidal chart cannot be used to directly convert tide readings made at the reference station to those observable at any other place as is the case with the range/time cotidal charts. With it however, tide at any point of interest in the area covered by the chart can be predicted at any time  $t$  using equation 2.25.

Interpolating between gauge stations has been the classical method of producing amplitude/phase cotidal charts. Presently a more meaningful method of producing this type of cotidal chart is through the solution of numerical schemes. Luther and Wunsch [1974] however used 350 sets of constants, obtained partly from the publications of the International Hydrographic Bureau (IHB) and partly from other investigators, to produce the cotidal charts for the central Pacific ocean which they claim are comparable with the numerical charts of Pekeris and Accad [1969] and Hendershott [1972].

### 3.2 Numerical Schemes

The various numerical schemes for the production of cotidal charts stem from various solutions of the Laplace tidal equations [Bye and Heath, 1975; Hendershott and Munk, 1970]

$$\frac{\partial u}{\partial t} - fv = \frac{g}{a \cos \phi} \cdot \frac{\partial(\xi - \bar{\xi})}{\partial \lambda} \quad (3.1)$$

$$\frac{\partial v}{\partial t} + fu = \frac{-g}{a} \cdot \frac{\partial(\xi - \bar{\xi})}{\partial \phi} \quad (3.2)$$

$$\frac{\partial \xi}{\partial t} + \frac{1}{a \cos \phi} \left( \frac{\partial u_Q}{\partial \lambda} + \frac{\partial v_Q}{\partial \phi} \cos \phi \right) = 0 \quad (3.3)$$

where  $\phi$ ,  $\lambda$  are the geodetic latitude and longitude respectively,

$u$ ,  $v$  are the latitudinal and longitudinal components of the fluid velocity,

$a$  is the earth mean radius,

$f(= 2\Omega \sin \phi)$  is the Coriolis parameter in which  $\Omega$  is the angular velocity of the earth,

$Q$  is the undisturbed depth of the ocean,

$\xi$  is the elevation of the sea surface above the undisturbed level, and

$\bar{\xi}(= V/g)$  is the equilibrium tide.

The Laplace tidal equations representing equations of motion, though they look simplified, are difficult to solve even in the case of uniform depth covering the globe. The early solutions were given by Lord Kelvin in 1845 and Hough in 1897 who replaced the Laplace power series in sine with an expansion in spherical harmonics thus regarding the earth's rotation as very small. In 1898, Lord Kelvin introduced the concept of  $f$ -plane approximation in which he considered the oscillations of the horizontal sheet of fluid of uniform depth rotating about its normal and this reduces the Laplace tidal equations to [Hendershott and Munk, 1970]

$$\frac{\partial u}{\partial t} - fv = -g \cdot \frac{\partial(\xi - \bar{\xi})}{\partial x}, \quad (3.4)$$

$$\frac{\partial v}{\partial t} + fu = -g \cdot \frac{\partial(\xi - \bar{\xi})}{\partial y}, \quad (3.5)$$

$$\frac{\partial \xi}{\partial t} + Q \left( \frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} \right) = 0 \quad . \quad (3.6)$$

in which  $x, y$  are the Cartesian coordinates in the plane of the fluid. Larsen [1977] used the  $f$ -plane solution to produce the cotidal charts for the Pacific ocean near the Hawaiian Islands. He approximated the Island as an elliptically shaped cylinder with the plane ocean taken to be tangent to the earth at the coordinates  $\phi_0 = 20.7^\circ\text{N}$  and  $\lambda_0 = 156.8^\circ\text{W}$  which corresponds to the coordinates of the centre of the elliptically shaped Island. On the plane ocean, he took the rectangular coordinate system with the  $X$ -axis eastwards and normal to the axis of the ridge formed by the island and the  $Y$ -axis northwards and parallel to the ridge axis and with the origin at the tangent point  $(\phi_0, \lambda_0)$ .

The boundary condition assumes that the velocity normal to the coast vanishes and free tide solutions are added in order to fit the observed tide at the boundary. The cotidal charts for the various constituents are constructed by mapping the amplitude and phase of the total tide, that is the resultant of the equilibrium tide, forced tide and free tide, as a function of the elliptic coordinates. The author evaluated the accuracy of the cotidal chart by comparing the observed tides at some locations with the values of tides predicted by the model. He observed that the plane wave model of the tides connect the tidal observations together in a simple way and thus

allows the tide to be interpolated between gauge stations and extrapolated into the ocean beyond the tidal sites.

Rossby in 1939 introduced the beta-plane approximation. In this, the Laplace tidal equations are written as in f-plane approximation but with the coriolis parameter made a linear function of  $y$ , namely

$$f = f_0 + \beta y \quad (3.7)$$

The variation of  $f$  with  $y$  corresponds to an expansion of the coriolis parameter about the latitude  $\phi_0$

$$2\Omega \sin \phi = 2\Omega \sin \phi_0 + \left(\frac{2\Omega}{a}\right)a(\phi - \phi_0)\cos \phi_0, \quad (3.8)$$

in which  $\beta$  is of the order  $\frac{2\Omega}{a}$ .

When  $\beta = 0$ , we then have f-plane approximation.

With the advent of large computers, the application of the method of finite differences to the tidal problems become popular. Freeman and Murty [1976] studied the cooscillating and independent tides in Hudson Bay and James Bay by applying the finite differences to solve the Laplace tidal equations. They linearised the equations of motion in spherical polar coordinates and vertically integrated retaining variable coriolis, pressure gradient, bottom stress and direct tidal potential terms. The equations thus solved in the model are

$$\frac{\partial u}{\partial t} = 2\Omega v \sin \phi - \frac{gh}{a \cos \phi} \cdot \frac{\partial \eta}{\partial \lambda} - \frac{T_{B\lambda}}{\rho} + \bar{F}_\lambda, \quad (3.8)$$

$$\frac{\partial v}{\partial t} = -2\Omega u \sin \phi - \frac{gh}{a} \cdot \frac{\partial \eta}{\partial \phi} - \frac{T_{B\phi}}{\rho} + \bar{F}_\phi, \quad (3.9)$$

$$\frac{\partial \eta}{\partial t} = \frac{1}{a \cos \phi} \left( \frac{\partial u}{\partial \lambda} + \frac{\partial v}{\partial \phi} \cos \phi \right), \quad (3.10)$$

where  $T_\beta$  is the bottom stress,  $\bar{F}_\lambda$ ,  $\bar{F}_\phi$  are the horizontal components of the tide generating force,  $\eta$  is the deviation of the water level from the mean tide level,  $h$  is the water depth and  $\rho$  is the density of water.

The cooscillating tide is modeled by setting the tide generating force terms to zero and specifying the free surface elevation across the mouth of the Hudson Bay by

$$\eta_k^1(\phi, \lambda) = H_k(\phi, \lambda) \cos(\omega_k t - g_k(\phi, \lambda)), \quad (3.11)$$

where  $\eta_k^1$  belongs to the constituent  $k$  at the open mouth boundary location and is referred to the mean tide level.

The independent tide is modeled by setting the normal velocity on the open mouth boundary to zero and specifying the tide generating force. For example, for  $M_2$

$$\bar{F}_2^\lambda = \frac{-48.8}{a} .gh \cos \phi \sin(\omega_m t + 2\lambda + \omega_m T), \quad (3.12)$$

$$\bar{F}_2^\phi = \frac{-48.8}{a} .gh \cos \phi \sin \phi \cos(\omega_m t + 2\lambda + \omega_m T), \quad (3.13)$$

and for  $K_1$

$$\bar{F}_1^\lambda = \frac{-28.5}{a} .gh \sin \phi \sin(\omega_k t + \lambda + \omega_k T), \quad (3.14)$$

$$\bar{F}_1^\phi = \frac{-28.5}{a} .gh(\sin^2 \phi - \cos^2 \phi) \cos(\omega_k t + \lambda + \omega_k T). \quad (3.15)$$

Here  $T$  is the number of hours from the Greenwich mean time to the local zone time. The linear form of bottom friction due to Heaps is used and is given as

$$T\beta_{\lambda} = \frac{PR}{h} U, \quad T\beta_{\phi} = \frac{PR}{h} V. \quad (3.16)$$

The authors used a rectangular grid of 15 and 10 minutes of arc in longitudinal and latitudinal directions respectively. The grids are drawn so that the fluid velocity components (U, V) are defined on the closed boundary locations and the water levels ( $\eta$ ) at the open boundary at the mouth of the Bay. In the formulation of the numerical scheme, central finite differences are used in both space and time. Using a leap-frog scheme, water levels ( $\eta$ ) are computed at even time steps (i.e.  $i = 2, 4, 6, 8, \dots$ ) and the horizontal flow components (U, V) computed at odd time steps (i.e.  $i = 1, 3, 5, 7, \dots$ ).

The numerical scheme is thus given by [Freeman and Murth, 1976]

$$\begin{aligned} \frac{U_{kj}^{i+1} - U_{kj}^{i-1}}{2\Delta t} &= 2\Omega V_{kj}^i \sin \phi_j - \frac{gh_{kj}}{a \cos \phi_j} \left( \eta_{k+1,j}^i - \eta_{k-1,j}^i \right) \\ &\quad - \frac{1}{\rho} T\beta_{\lambda k,j}^{i-1} + \bar{F}_{\lambda k,j}^i, \end{aligned} \quad (3.17)$$

$$\begin{aligned} \frac{V_{k,j}^{i+1} - V_{k,j}^{i-1}}{2\Delta t} &= -2\Omega U_{k,j}^i \sin \phi_j - \frac{gh_{k,j}}{a} \left( \eta_{k,j+1}^i - \eta_{k,j-1}^i \right) \\ &\quad - \frac{1}{\rho} T\beta_{\phi i,j}^i + \bar{F}_{\phi k,j}^i, \end{aligned} \quad (3.18)$$

$$\begin{aligned} \eta_{k,j}^{i+1} - \eta_{k,j}^{i-1} &= -\frac{1}{a \cos \phi_j} \left( \frac{U_{k+1,j}^i - U_{k-1,j}^i}{2\Delta \lambda} \right. \\ &\quad \left. + \frac{V_{k,j+1}^i \cos \phi_{j+1} - V_{k,j-1}^i \cos \phi_{j-1}}{2\Delta \phi} \right), \end{aligned} \quad (3.19)$$

and the output of the computations are  $U$ ,  $V$  and  $\eta$  as functions of time. From these parameters, the current ellipses and the co-phase and co-amplitude lines are constructed.

In numerical schemes, the problem generally posed is to solve the Laplace tidal equations in their primitive form or after elimination of one or two dependent variables with prescribed boundary conditions. For example

- (i) Vanishing normal velocity at coast lines  
[Pekeris and Accad, 1967],
- (ii) Specified or observed values of the constituents at the coastal stations only [Hendershott, 1966],
- (iii) Specified or observed values of the constituents at selected coastal and island stations plus vanishing normal velocity at the remaining coastal boundary points [Larsen, 1977].

### 3.3 Uses of Cotidal Charts

Cotidal charts are found useful in many situations.

They are useful in the study of the impact of large engineering structures on the tidal regime, for example, the proposed tidal power project on the Bay of Fundy in Eastern Canada [Atlantic Tidal Power Engineering and Management Committee Report, 1969; Garrett and Greenberg, 1976].

They are indispensable in navigation especially when deep draught ships have to navigate through a complex

estuary where drying sand banks alternate with deeps such as that obtained in the port of London [White, 1971]. Here deep draught tankers navigate to Thameshaven and Coryton to evacuate oil from the principal oil refineries. In such a situation the pilot and the captain of the vessel would want the information on

- (i) the critical depths in the channel at chart datum,
- (ii) the points along the track where these critical depths occur,
- (iii) the times the tidal heights at these points would be sufficient for safe passage of a vessel with a particular draught,
- (iv) the latest times along the route that the passage depths are available.

If the underkeel clearance is not so critical, this information can easily be obtained using cotidal charts and appropriate up to date navigation charts and tide tables.

If the underkeel clearance is critical, the use of cotidal charts is supplemented by several radio linked tide gauges.

The application of prime concern here is the use of cotidal charts for the reduction of sounding data. As was shown previously, all depth measurements are reduced to the chart datum; therefore the height of tide at time  $t$  must be subtracted from the depth sounded at the time  $t$ . This implies that we should observe tides at the same time we



take our soundings. If we are working on the coast or on the inland tidal waters, it is possible to establish tide gauges close to the sounding area and observe tides at the same time. If we are involved with extensive sounding offshore, the possibilities of observing tides close to the sounding area are remote. It becomes more feasible to do the tidal reductions using predicted tides, and when this is the case, the use of cotidal charts become convenient.

Range/time cotidal charts can be used in which case we only need to observe or predict tides at the reference station and then obtain the equivalent at the desired locations, or, we can use amplitude/phase cotidal charts and predict the tides at the desired locations independent of a reference station. Finally, a combination of the two approaches can be used.

The Canadian Hydrographic Service has done some automated tidal reductions using digitized range/time cotidal charts of the Hudson Bay and the Lower St. Lawrence River [Tinney, 1977]. In these schemes, the cotidal charts were digitized by breaking the survey area into equal size blocks based on lines of latitude and longitude and approximating the boundaries of the cotidal zones with the edges of those blocks. Those digitizations were coded and stored in the computer. To locate a particular block and retrieve the cotidal values, the geodetic coordinates ( $\phi$ ,  $\lambda$ ) of the position of the sounding were used.

The choice of the size of the blocks would obviously

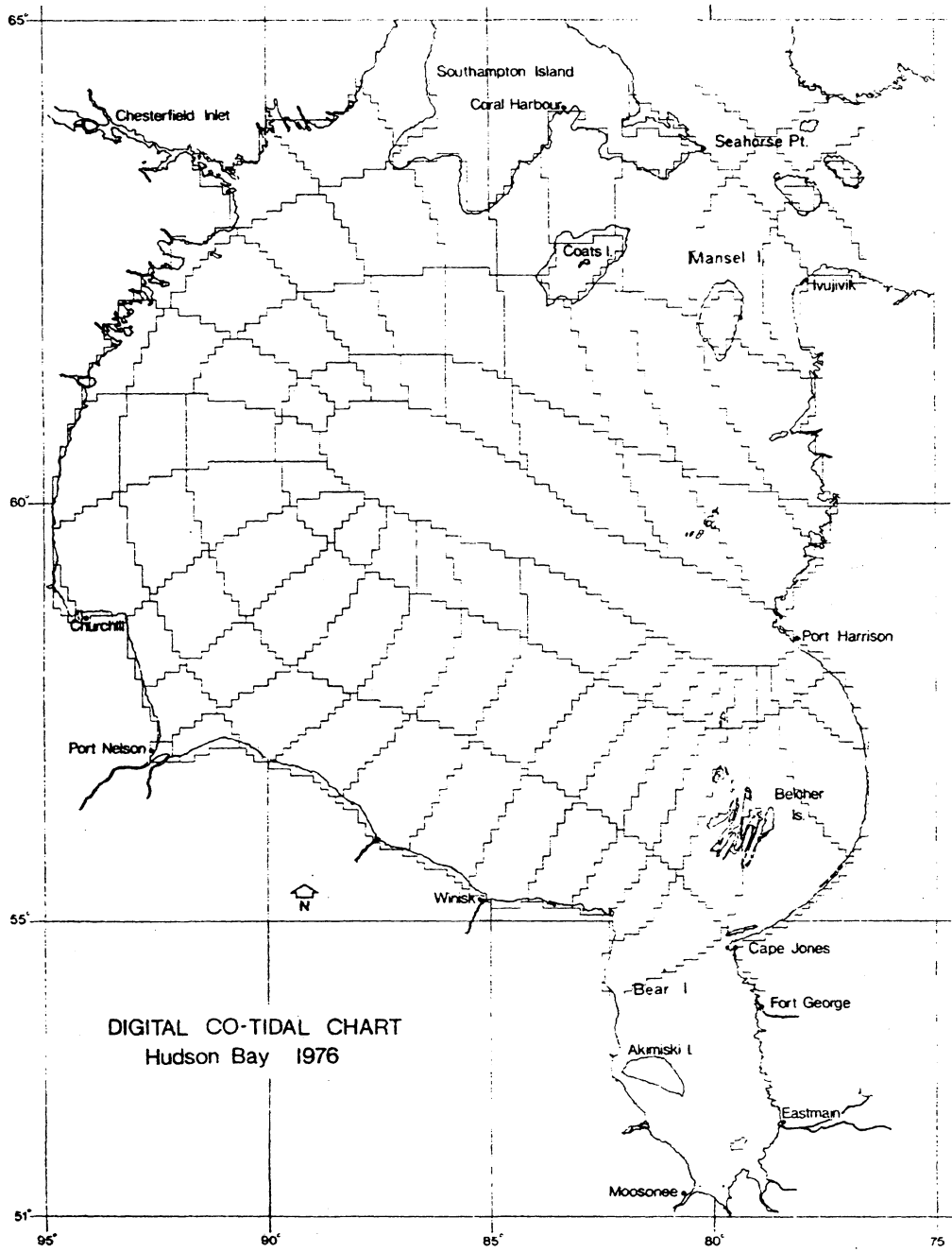


Figure 3-3

depend on the amount of computer space available and the accuracy requirements. With smaller size blocks, the zone boundaries would be better approximated but more computer space would be required. Figure 3-3 shows the digital breakdown of the cotidal chart used for the Hudson Bay. Block sizes of 5' latitude and 10' longitude were used giving a total of 13,986 blocks dividing the Bay into 93 reduction zones. The tide station at Churchill served as the reference station for the cotidal chart and during the survey, the predicted heights from the reference station were used instead of the observed heights. However, in the survey of the Lower St. Lawrence River with Pointe-an-Père as the reference station, observed tides were used.

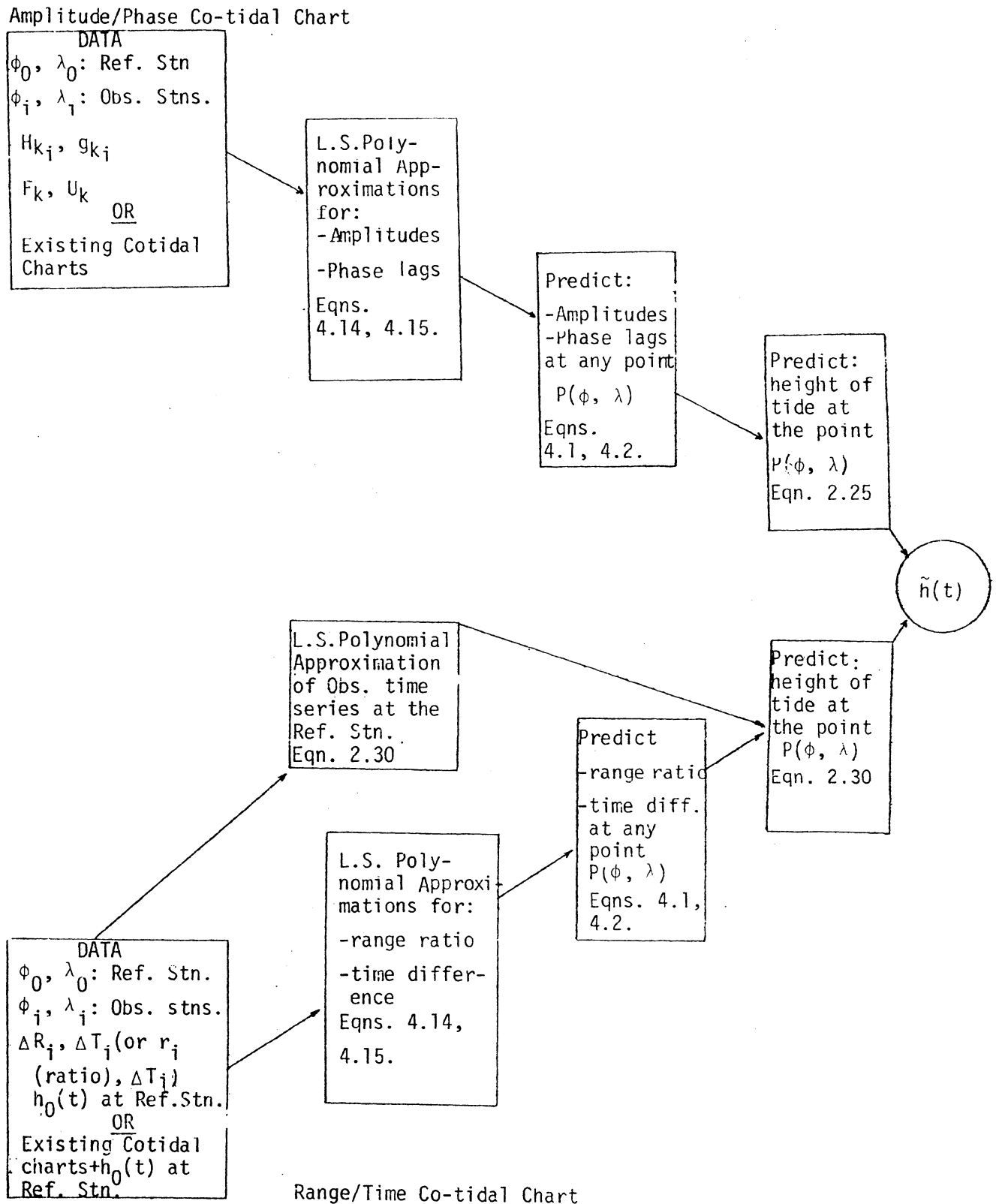
## IV THE PROPOSED ANALYTICAL SCHEME

### 4.0 Introduction

The proposed analytical scheme is aimed at achieving automated tidal reductions using little computer space and time and with advantageous accuracy and flexibility. Figure 4-1 illustrates the proposed scheme in a flow-chart. It shows that we can work with amplitude/phase cotidal model or range/time cotidal model. The same objective is achieved using either model but it does not necessarily mean that the same degree of accuracy and flexibility is attained. Basically the data requirements for either are the same except that with amplitude/phase cotidal model, the amplitude  $H_k$  and the phase lag  $g_k$  for each constituent  $k$  we wish to take into account and at each observation station are required. With the range/time cotidal model, we require the mean range ratios and the mean differences of the times of occurrence of high and low waters between each tide gauge station to be considered and a reference gauge station. With the range/time cotidal model, we have the option of carrying out the tidal reduction based on the observed tides or on the predicted tides at the reference station.

In each case, the aim is to produce an analytical cotidal model using observed data or existing cotidal charts. The analytical model could then be stored conveniently in a computer so that when observed sounding

Figure 4-1 The Proposed Scheme - Flow Chart



data are input, the output would be reduced soundings. The theory and mathematical models for the two approaches are basically the same. In Section 4.1 of this chapter, the mathematical models are discussed, and in Section 4.2 the data requirements are explained explicitly.

#### 4.1 Models

Earlier, it was shown that the tides are functions of time and position on the surface of the earth and that the tidal characteristics, that is, the amplitude  $H_k$  and the phase lag  $g_k$  for the constituent  $k$  are constant for a place. These constants can be estimated by performing harmonic or response analysis of a long period tidal records. Knowing the estimated tidal constants for a place, the tide at the place can be predicted at any time  $t$ .

Now suppose we consider a section of a body of tidal water, not so extensive in area and where the constants  $H_k$  and  $g_k$  are defined at a reference station whose geodetic coordinates are  $(\phi_0, \lambda_0)$ , and at several other points  $P_j(\phi_j, \lambda_j)$  within the area. We can define mathematically surfaces that can describe the distribution of those constants with reference to the primary station. The aim is to approximate, in the Least Squares sense, the amplitude and phase lag fields by surfaces described by two dimensional algebraic polynomials. The coefficients of these polynomials are determined in such a way as to fit the observed data in the Least Squares sense. Using this

technique, the amplitudes  $H_k$  and the phase lag  $g_k$  can be predicted at any point of interest  $P_i(\phi_i, \lambda_i)$  within the area by the polynomials

$$\tilde{\Delta}H_k(x_i, y_i) = \sum_{j=0}^{\ell} C_j^H \psi_j(x_i, y_i). \quad (4.1)$$

$$\tilde{\Delta}g_k(x_i, y_i) = \sum_{j=0}^{\ell} C_j^g \psi_j(x_i, y_i), \quad (4.2)$$

where  $\tilde{\Delta}H_k(x_i, y_i)$  and  $\tilde{\Delta}g_k(x_i, y_i)$  are the predicted differences in amplitude and phase lag respectively for the constituent  $k$  between the reference station and the point  $i$ ,  $C_j^H$  and  $C_j^g$  are the coefficients of the polynomials,  $\psi(x_i, y_i)$  are base functions (two dimensional) of the approximating polynomials, and  $\ell$  is the number of base functions. The selection of the prescribed functions  $\psi$  can be, from the theoretical point of view, purely arbitrary. The sufficient and necessary condition for the prescribed functions  $\psi \equiv \{\psi_1, \psi_2 \dots \psi_\ell\}$  to create a base is that they are linearly independent on the functional space ( $G_m$ ). If and only if  $\psi$  is a base can the coefficients of the best fitting polynomial be uniquely determined [Vaniček and Wells, 1972].

Even though the position of a point may be expressed in terms of geodetic coordinates  $(\phi_i, \lambda_i)$ , it is more convenient to work with local orthogonal coordinates  $(x_i, y_i)$ . The relationship between the two systems is defined as

$$x_i = R_0(\phi_i - \phi_0), \quad (4.3)$$

$$y_i = R_0 \cos \phi_0(\lambda_i - \lambda_0) . \quad (4.4)$$

where  $R_0$  is the mean radius of curvature of the earth computed at the reference station and is given by [Krakiwsky and Wells, 1971]

$$R_0 = \sqrt{M_0 N_0} \quad (4.5)$$

in which

$$M_0 = a(1 - e^2)/(1 - e^2 \sin^2 \phi_0)^{3/2}, \quad (4.6)$$

and

$$N_0 = a/(1 - e^2 \sin^2 \phi_0)^{1/2}. \quad (4.7)$$

The first eccentricity squared is

$$e^2 = (a^2 - b^2)/a^2, \quad (4.7b)$$

and for the Clarke 1866 ellipsoid, the semi-major axis  $a = 6378.2064$  km, while the semi-minor axis  $b = 6356.5838$  km.

Regarding the choice of base functions, we can use mixed algebraic functions which are particularly simple to deal with [Nassar and Vaniček, 1975], namely,

$$\psi = \{x^\ell y^j\}, \quad (\ell, j = 0, 1, 2 \dots n) \quad (4.8)$$

where  $n$  is the degree of the polynomial. Equation 4.1 and 4.2 can now be rewritten as

$$\tilde{\Delta}H_k(x_i, y_i) = \sum_{\ell, j=0}^n C_j^H x_i^\ell y_i^j, \quad (4.9)$$

$$\tilde{\Delta}g_k(x_i, y_i) = \sum_{\ell, j=0}^n C_j^g x_i^\ell y_i^j. \quad (4.10)$$

The problem is to solve for the coefficients  $C_j^H$  and  $C_j^g$  of the polynomials. The number of coefficients  $U$  to be solved for is determined from the relation

$$U = (n + 1)d. \quad (4.11)$$



where  $n$  is the degree of the polynomial and  $d$  is the dimensionality of the base functions.

#### 4.2.1 Least Squares Solution of the Models

To determine the unknown coefficients  $C_j^H$  and  $C_j^g$  of the models represented by equations 4.9 and 4.10, observation equations can be written for each data point  $i$  where the amplitude difference  $\Delta H_k$  and the phase lag difference  $\Delta g_k$  referred to a reference station are known. The equations are

$$\tilde{\Delta H}_k(x_i, y_i) + V_{H_{ki}} = \Delta H_k(x_i, y_i), \quad (4.12)$$

$$\tilde{\Delta g}_k(x_i, y_i) + V_{g_{ki}} = \Delta g_k(x_i, y_i), \quad (4.13)$$

where  $\tilde{\Delta H}_k$  and  $\tilde{\Delta g}_k$  are the predicted values,  $V_{H_{ki}}$  and  $V_{g_{ki}}$  are the residuals of observations, and the terms on the right hand side ( $\Delta H_k$  and  $\Delta g_k$ ) are the known or observed values. Substituting equations 4.9 and 4.10 into equations 4.12 and 4.13 yields

$$\sum_{\ell, j=0}^n C_{\ell j}^H x_i^\ell y_i^j + V_{H_{ki}} = \Delta H_k(x_i, y_i), \quad (4.14)$$

$$\sum_{\ell, j=0}^n C_{\ell j}^g x_i^\ell y_i^j + V_{g_{ki}} = \Delta g(x_i, y_i). \quad (4.15)$$

Putting equations 4.14 and 4.15 in matrix form we have

$$\begin{matrix} A & C^H & + & V_H & = & L_H \\ mxu & ux1 & & mx1 & & mx1 \end{matrix}, \quad (4.16)$$

$$\begin{matrix} A & C^g & + & V_g & = & L_g \\ mxu & ux1 & & mx1 & & mx1 \end{matrix}. \quad (4.17)$$

It is pertinent to note here that equations 4.16 and 4.17 are the same as the observation equations for a parametric case

in the least squares adjustments. The parametric least squares adjustment differs only in purpose and notations from the least squares approximation of a function (F) defined on a discrete or compact domain (M) [Vaniček and Wells, 1972]. The purpose of the least squares approximation is to find an approximating polynomial ( $P_n$ ) for a given function or for a given set of functional values. The purpose of the least squares adjustment is to find the least squares statistical estimates of unknown parameters which are related to the observed values by linear (or linearized) mathematical models.

The matrix A is known as Vandermonde's design matrix and is given by

$$A_{m \times u} \equiv \begin{bmatrix} \psi_0(x_1^0 y_1^0), \psi_1(x_1^0 y_1^1), \psi_2(x_1^0 y_1^2), \dots, \psi_u(x_1^n y_1^n) \\ \psi_0(x_2^0 y_2^0), \psi_1(x_2^0 y_2^1), \psi_2(x_2^0 y_2^2), \dots, \psi_u(x_2^n y_2^n) \\ \psi_0(x_m^0 y_m^0), \psi_1(x_m^0 y_m^1), \psi_2(x_m^0 y_m^2), \dots, \psi_u(x_m^n y_m^n) \end{bmatrix} \quad (4.18)$$

$C^H$  and  $C^g$  are the vectors of coefficients.  $V_H$  and  $V_g$  are the vectors of residuals of the observations  $L_H$  and  $L_g$ .  $L_H$  and  $L_g$  are the vectors of observed values (or the functional values) at the discrete points  $i$ . The solution of the system of equations given by 4.16 and 4.17 for the coefficients, using least squares approximation methodology [Vaniček and Wells, 1972; Christodoulidis, 1973; Balogun, 1977; Appendix I] is given by

$$\hat{C} = N^{-1}U, \quad (4.19)$$

where  $N$  is the Gram's matrix defined by

$$N_{uxu} \equiv [A^T P A] \equiv \begin{bmatrix} \langle \psi_0 \psi_0 \rangle, \langle \psi_0 \psi_1 \rangle & \cdots & \langle \psi_0 \psi_u \rangle \\ \langle \psi_1 \psi_0 \rangle, \langle \psi_1 \psi_1 \rangle & \cdots & \langle \psi_1 \psi_u \rangle \\ \vdots & \vdots & \vdots \\ \langle \psi_u \psi_0 \rangle, \langle \psi_u \psi_1 \rangle & \cdots & \langle \psi_u \psi_u \rangle \end{bmatrix}, \quad (4.20)$$

and

$$U_{ux1} \equiv A^T P L \equiv \langle L, \psi_i \rangle. \quad (4.21)$$

The sign  $\langle \rangle$  indicates a scalar product [Appendix I].

Since our prescribed functions form a base, the Gram's determinant must be different from zero and must have an inverse.

The solution for the residual vector is given by

$$\hat{V} = \hat{A}C - L. \quad (4.22)$$

The associated variance covariance matrix for the coefficients is given by

$$\hat{\Sigma}_{\hat{C}} = \hat{\sigma}^2 N^{-1}, \quad (4.23)$$

where  $\hat{\sigma}^2$  is the a posteriori variance factor given by

$$\hat{\sigma}^2 = \hat{V}^T P \hat{V} / df, \quad (4.24)$$

in which  $df$  represents the degree of freedom given by

$$df = m - u. \quad (4.25)$$

$P$  is the weight matrix

$$P = \hat{\Sigma}_L^{-1} = \text{Diag} \left[ \frac{1}{\sigma_{L_1}^2}, \frac{1}{\sigma_{L_2}^2}, \cdots, \frac{1}{\sigma_{L_m}^2} \right] \quad (4.26)$$

where  $\sigma_L$  is the standard error of the observables. The weight matrix is diagonal when we are dealing with statistically independent observables, that is, the observations are assumed uncorrelated.

For statistical reasons, we may wish to work with orthogonal bases, and usually the base  $\psi$  is not an orthogonal one. Schmidt's orthogonalization process [Appendix I] may be applied to obtain an orthogonal base  $\psi^*$ . Using an orthogonal base, the normal equation is

$$A^{*T} P A^* \hat{C}^* = A^{*T} P L. \quad (4.27)$$

Again setting

$$A^{*T} P A^* = N^*,$$

and

$$A^{*T} P L = U^*,$$

we have that

$$\hat{C}^* = N^{*-1} U^*. \quad (4.28)$$

$A^*$  is the Vandermonde's design matrix obtained using the orthogonal base.  $\hat{C}^*$  is a vector of Fourier coefficients,  $N^*$  is the Gram's matrix, this time diagonal because we are dealing with orthogonal base functions and is given by

$$N^*_{uxu} \equiv \begin{bmatrix} \langle \psi_0^* \psi_0^* \rangle & 0 & 0 \\ 0 & \langle \psi_1^* \psi_1^* \rangle & 0 \\ 0 & 0 & \langle \psi_u^* \psi_u^* \rangle \end{bmatrix}, \quad (4.29)$$

and  $U^*$  is given by

$$\begin{array}{l}
 U^* \\
 \text{uxl}
 \end{array}
 \equiv
 \begin{bmatrix}
 \langle \psi_0^* & L \rangle \\
 \langle \psi_1^* & L \rangle \\
 \vdots \\
 \langle \psi_u^* & L \rangle
 \end{bmatrix}
 . \quad (4.30)$$

The associated variances are given by

$$\sum \hat{C}_i^* = \hat{\sigma}^{2*} N^{*-1} , \quad (4.31)$$

where

$$\hat{\sigma}^{2*} = \hat{V}^{*T} P \hat{V}^* / df . \quad (4.32)$$

The solution of normal equation becomes trivial as the normal equation matrix  $N^*$  (Grams matrix) is diagonal and each Fourier coefficient can be solved for independently.

We subject the Fourier coefficients to statistical screening by comparing each coefficient against  $j$  times its standard error. [Christodoulidis, 1973], that is, if

$$|\hat{C}_i^*| < j \sigma_{\hat{C}_i^*} , \quad (4.33)$$

then  $\hat{C}_i^*$  is statistically insignificant at that level and is discarded.  $j$  takes the values 1, 2 or 3 depending upon what level of significance of their standard deviations we wish to test the coefficients. The discarded Fourier coefficients are set equal to zero. Once the appropriate Fourier coefficients are discarded, the residuals, the variance factor and the variances are recomputed using only the accepted coefficients. The residuals are given by

$$\hat{V}^{*1} = A^* \hat{C}^* - L . \quad (4.34)$$

The a posteriori variance factor is recomputed by

$$\hat{\sigma}^{2*1} = \hat{V}^{*1T} P \hat{V}^{*1} / df^1, \quad (4.35)$$

where

$$df^1 = m - u + d,$$

in which  $d$  represents the number of Fourier coefficients discarded. The new variances are

$$\sum \hat{C}^{*1} = \hat{\sigma}^{2*1} N^{*-1}. \quad (4.36)$$

Using the transformation matrix (see Appendix I)

$$B_{u \times u} = \begin{bmatrix} 1 & \beta_{12} & \beta_{13} & \cdots & \beta_{1u} \\ 0 & 1 & \beta_{23} & \cdots & \beta_{2u} \\ \vdots & \vdots & \vdots & \ddots & \vdots \\ 0 & 0 & 0 & \cdots & 1 \end{bmatrix} \quad (4.37)$$

and the remaining statistically significant Fourier coefficients, the original coefficients are computed by

$$\hat{C} = B \hat{C}^* . \quad (4.38)$$

The correct number of original coefficients are obtained even though we are solving for them using fewer number of Fourier coefficients. If, however, the last Fourier coefficients are the ones discarded, a fewer number of original coefficients will be recovered. The variance-covariance matrix of the original coefficients can be computed using the variance-covariance law, namely,

$$\sum \hat{C} = B \sum \hat{C}^{*1} B^T, \quad (4.39)$$

where  $\sum \hat{C}^{*1}$  is given by equation 4.36.

Once we have computed the coefficients of the original polynomials and their variance-covariance matrix from the statistically significant Fourier coefficients, statistically significant surfaces which describe the distributions of amplitudes and phase lags (or range ratios and time differences) in the area of interest have been obtained. Analytical cotidal models for amplitudes and phase lags (or range ratios and time differences) have thus been obtained. With the analytical models, the values of the amplitude and phase lags (or range ratio and time lags) can be predicted for any point  $P_i(\phi_i, \lambda_i)$  in the area using equations 4.1 and 4.2. The prediction variance covariance matrix is given by

$$\hat{\Sigma}_{\tilde{H}} = J \hat{\Sigma}_{\hat{C}} J^T \quad (4.40)$$

in which J is Jacobian of transformation defined by A matrix.

### 4.3 Data Requirements and Reduction Algorithms

#### 4.3.1 Amplitude/Phase Cotidal Model

As previously noted, to produce cotidal models for amplitudes and phase lags, we need to define the amplitudes and the phase lags of each constituent frequency at a reference station and at several other observation stations adequately distributed in an area of interest. Working with four major constituents, eight analytical models are needed to describe the tidal characteristics of the area. For a fair estimate of the amplitudes and the phase lags, the tidal analysis must be made from

369 days of tidal records, and for a barely acceptable estimate, observation should cover a period of 29 days. The more observations added in the analysis, the better will be the estimate of the harmonic constants.

It may not be easy to adequately distribute observing stations and obtain sufficient data to enable the production of a desired analytical model. An alternative is to use cotidal charts, produced from the numerical schemes such as those described in III, Section 3.2, as a source of data. The cotidal charts are digitized as mentioned in Section 3.3 and the digitized values are used in the least squares polynomial approximations to produce the analytical cotidal models. As a check on the compatibility of the analytical models and the original chart, the area is grided at close intervals and the amplitudes and phase lags predicted at the grid intersections using equations 4.1 and 4.2. The co-amplitude and co-phase curves can then be easily drawn in.

If the amplitude/phase cotidal models are being used for the reduction of soundings, the reduction algorithms can be summarized in steps as follows:

- (i) At each sounding location  $i$ , the depth ( $D_i$ ), the time ( $t$ ) and the geodetic coordinates ( $\phi_i, \lambda_i$ ) are observed.
- (ii) With the observed geodetic coordinates ( $\phi_i, \lambda_i$ ), the amplitudes and phase lags



of the constituents being used can be predicted using the analytical models.

(iii) Using the tide prediction approach as described in II Section 2.2 and the predicted amplitudes and phase lags from (ii) above, the height of tide  $h_i(t)$  at the sounding location above chart datum are predicted.

(iv) The sounding reduced to the chart

$$\text{datum is } d_i = D_i - h_i(t) \quad (4.41)$$

#### 4.3.2 Range/Time Cotidal Model

Some assumptions must be made at the outset for this model. Considering a body of water of relatively small extent, such that one can safely assume that the meteorological variables in the area are not remarkably different from place to place, it can be further assumed that given any two points  $A(\phi, \lambda)$  and  $B(\phi, \lambda)$  in the area, the tides at A bear constant relationships with the tides at B. Those relationships will change when there are marked topographical changes due, for example, to erosion, engineering structures, which tend to change the pattern of the propagation of tidal waves. If we establish the relationship existing between a reference station and any other point, it is possible to predict with some degree of certainty the tides at that other point from the observed (or predicted) tides at the reference station.

The relationships between the tides at any two stations can be established from the ratio of their ranges and the difference in the times of occurrence of high and low waters. In other words, it is assumed that the unwanted noise has perturbed observations equally so that when the range ratios and time differences are determined, the unwanted noise is eliminated.

To produce range ratios and time lags cotidal models, we require

- (i) the mean range  $R_{m0}$  at the reference station and the mean ranges  $R_{mj}$  at discrete points  $(\phi_j, \lambda_j)$ ; the range ratios are then given as

$$r_j = R_{mj}/R_{m0} \quad (4.42)$$

- (ii) the mean time differences between the times of high and low waters at the reference station and at the discrete points given in minutes of time.

If the sounding reduction is to be done with range/time analytical cotidal model, the reduction algorithms can be summarized in the following steps:

- (i) The tide is observed at the reference station to cover the time interval  $M$  (the soundings are also performed within the same interval of time). A least squares approximation of the observed series at the reference

station is done so that at any time  $t$  in the interval, the height of tide can be predicted.

- (ii) At each sounding location  $i$ , the depth ( $D_i$ ), the time ( $t$ ) and the geodetic coordinates ( $\phi_i, \lambda_i$ ) are observed.
- (iii) With the observed geodetic coordinates ( $\phi_i, \lambda_i$ ), the range ratio ( $r_i$ ) and time difference (correction to time) are predicted using the analytical models.
- (iv) Using the corrected time at the reference station and the approximating polynomial from step (i) above, the height of tide ( $h_0(t)$ ) at the reference station is predicted.
- (v) The height of tide at the observed location  $i$  is computed from the relation

$$h_i(t) = h_0(t) \times r_i . \quad (4.43)$$

- (vi) The reduced sounding is

$$d_i = D_i - h_i(t) . \quad (4.44)$$

It is more convenient and simple to work with range/time cotidal models because (i) unlike the amplitude/phase models where  $2 \times NCON$  ( $NCON$  is the number of constituents used) analytical models are needed to describe the tides, only two models are needed to completely describe the tides, (ii) working with range/time cotidal models allows

us to use the observed tides at the reference station to reduce soundings instead of the predicted tides .

## V TEST COMPUTATIONS AND THE RESULTS

### 5.0 Data

To test the proposed analytical scheme, there was unfortunately no adequate data immediately available. However, the tidal information for secondary ports on the Bay of Fundy, published in the Canadian Tides and Current Tables, 1978 by the Canadian Hydrographic Service was minimally adequate for testing the analytical range/time cotidal models. This tidal information is given with reference to the Port of Saint John. In Table 5-1, the data as extracted are tabulated for 35 secondary stations (Figure 5-1).

The predicted tides for the Port of Saint John from January 1-15, 1978, were extracted from the same Canadian Tides and Current Tables, 1978 and treated as observed tides in the computations. The zero hour of the day the observation started is taken as the origin of time and times are given in hours from the origin of time. The observations are treated such that the period of the sounding exercise is covered, in other words, it is assumed that the tides were observed at Saint John throughout the period of the sounding. In Table 5-2, the tides as supposedly observed are tabulated and from Table 2-1 the following 7 major constituent frequencies are used.

| <u>Symbol</u>  | <u>Frequency (deg./hr)</u> |
|----------------|----------------------------|
| M <sub>2</sub> | 28.984104                  |
| S <sub>2</sub> | 30.00000                   |

| Index No. | Location Name   | Zone Time(ZT) | Latitude |    | Longitude |    | Mean Range | Range Ratio(r) | Mean Time Diff.(min) | Remark   |
|-----------|-----------------|---------------|----------|----|-----------|----|------------|----------------|----------------------|----------|
| 0065      | Saint John      | +4            | 45       | 16 | -66       | 04 | 25.10      | 1.0            | 0.0                  | Ref. St. |
| 0001      | Outer Wood Isl. | +4            | 44       | 36 | -66       | 48 | 16.60      | 0.6614         | -28.5                | *        |
| 0015      | Welshpool       | +4            | 44       | 53 | -66       | 57 | 16.90      | 0.6733         | + 5.0                | *        |
| 0040      | St. Andrews     | +4            | 45       | 04 | -67       | 03 | 22.60      | 0.9004         | +15.5                | *        |
| 0060      | Partridge Isl.  | +4            | 45       | 14 | -66       | 03 | 25.00      | 0.9960         | -10.0                | *        |
| 0129      | St. Martins     | +4            | 45       | 21 | -65       | 32 | 30.15      | 1.2012         | + 9.0                | *        |
| 0140      | Herring Cove    | +4            | 45       | 34 | -64       | 58 | 33.25      | 1.3247         | +19.0                | *        |
| 0150      | Cape Enrage     | +4            | 45       | 36 | -64       | 47 | 35.40      | 1.4104         | +17.0                | *        |
| 0160      | Grindstone Isl. | +4            | 45       | 44 | -64       | 37 | 38.30      | 1.5359         | +20.0                |          |
| 0170      | Hopewell Cape   | +4            | 45       | 51 | -64       | 35 | 39.90      | 1.5896         | +19.0                |          |
| 0190      | Pecks Point     | +4            | 45       | 45 | -64       | 29 | 38.70      | 1.5418         | +19.0                |          |
| 0215      | Joggins Wharf   | +4            | 45       | 41 | -64       | 28 | 38.15      | 1.5199         | +18.5                |          |
| 0225      | Cape Capstan    | +4            | 45       | 28 | -64       | 51 | 33.05      | 1.3167         | +11.0                | *        |
| 0235      | West Advocate   | +4            | 45       | 21 | -64       | 49 | 32.90      | 1.3107         | - 1.0                | *        |
| 0240      | Cape D'or       | +4            | 45       | 18 | -64       | 47 | 36.55      | 1.4562         | +16.5                | *        |
| 0245      | Port Greville   | +4            | 45       | 40 | -64       | 56 | 36.70      | 1.4622         | +30.0                |          |
| 0247      | Diggent River   | +4            | 45       | 24 | -64       | 27 | 39.50      | 1.5737         | +33.0                |          |
| 0250      | Cape Sharp      | +4            | 45       | 22 | -64       | 23 | 37.95      | 1.5120         | +48.5                |          |
| 0260      | Five Isl.       | +4            | 45       | 23 | -64       | 08 | 43.05      | 1.7151         | +56.0                |          |
| 0270      | Burnstooat Head | +4            | 45       | 18 | -63       | 48 | 44.30      | 1.7649         | +67.0                |          |
| 0285      | Avon Port       | +4            | 45       | 06 | -63       | 13 | 45.05      | 1.7948         | +32.5                |          |
| 0290      | Cape Blomidon   | +4            | 45       | 16 | -64       | 21 | 29.80      | 1.1873         | +46.0                |          |
| 0300      | Scots Bay       | +4            | 45       | 19 | -64       | 26 | 37.10      | 1.4781         | +14.5                |          |

Table 5-1 Bay of Fundy - Tidal Information on Secondary Port.

| Index No. | Location Name     | Zone Time (ZT) | Latitude | Longitude | Mean Range | Range Ratio (r) | Mean Time Diff.(min) | Remark |
|-----------|-------------------|----------------|----------|-----------|------------|-----------------|----------------------|--------|
| 0305      | Baxter Harbour    | +4             | 45 14    | -64 31    | 37.4       | 1.4900          | +12.0                | *      |
| 0312      | Ile Haute         | +4             | 45 15    | -65 00    | 34.15      | 1.3606          | 0.0                  | *      |
| 0315      | Margaretsville    | +4             | 45 03    | -65 04    | 31.75      | 1.2649          | -12.0                | *      |
| 0320      | Parkers Cove      | +4             | 44 48    | -65 32    | 26.60      | 1.0598          | -14.0                | *      |
| 0325      | Digby             | +4             | 44 38    | -65 45    | 25.25      | 1.0060          | - 9.0                |        |
| 0330      | Deep Cove         | +4             | 44 24    | -65 50    | 24.00      | 0.9562          | -15.5                | *      |
| 0335      | Sand Cove         | +4             | 44 30    | -66 06    | 21.15      | 0.8426          | -18.0                | *      |
| 0336      | East Sandy Narro. | +4             | 44 29    | -66 05    | 19.10      | 0.7610          | -37.0                | *      |
| 0337      | Tiverton          | +4             | 44 23    | -66 13    | 17.45      | 0.6952          | -45.0                |        |
| 0340      | West Port         | +4             | 44 16    | -66 21    | 18.10      | 0.7211          | -34.0                | *      |
| 0345      | Lighthouse Cove   | +4             | 44 15    | -66 24    | 17.90      | 0.7131          | -34.0                | *      |
| 0353      | Church Point      | +4             | 44 20    | -66 07    | 18.10      | 0.7211          | +18.0                | *      |
| 0355      | Meteghan          | +4             | 44 12    | -66 10    | 16.90      | 0.6733          | +18.0                | *      |

\* Data used in test computations

Table 5-1 (cont'd).

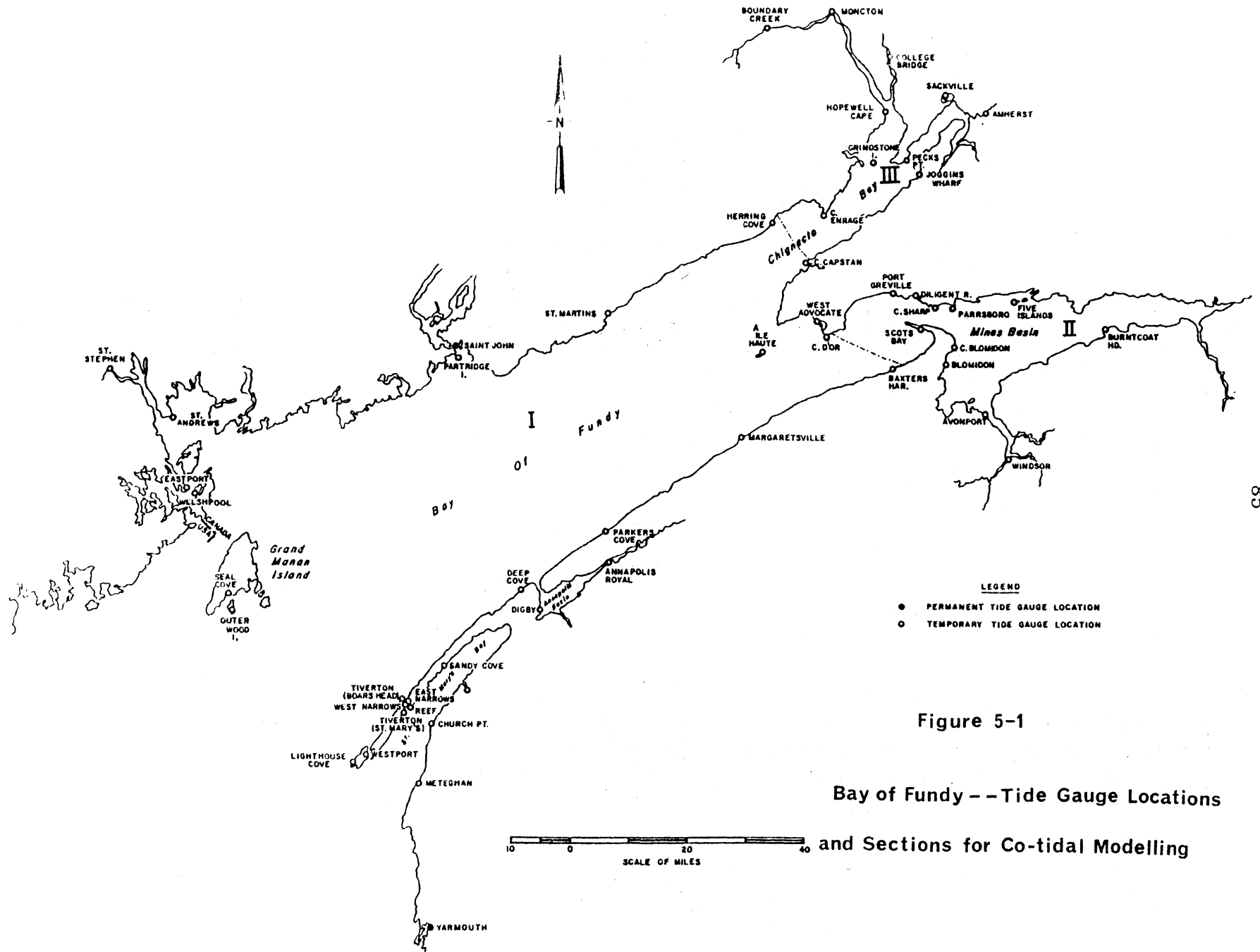


Figure 5-1  
 Bay of Fundy -- Tide Gauge Locations  
 and Sections for Co-tidal Modelling

00



Station: Saint JohnTime Zone: + 4Coords.: Lat. = 45 16'N  
Long. = 66 04'WDate: Jan. 1-15, 1978

| Time (Hrs) | Height(m) | Time (Hrs) | Height(m) | Time (Hrs) | Height(m) |
|------------|-----------|------------|-----------|------------|-----------|
| 03.833     | 7.4       | 128.500    | 8.1       | 253.167    | 8.4       |
| 10.250     | 1.3       | 134.833    | 0.6       | 259.333    | 0.2       |
| 16.250     | 7.3       | 141.083    | 7.7       | 265.500    | 8.0       |
| 22.350     | 1.2       | 147.333    | 0.8       | 271.917    | 0.6       |
| 28.667     | 7.4       | 153.417    | 8.3       | 278.000    | 8.1       |
| 35.00      | 1.2       | 159.833    | 0.3       | 284.333    | 0.4       |
| 41.250     | 7.3       | 166.083    | 7.8       | 290.500    | 7.8       |
| 47.250     | 1.2       | 172.167    | 0.6       | 296.667    | 0.8       |
| 53.500     | 7.6       | 178.25     | 8.4       | 302.833    | 7.8       |
| 59.917     | 1.2       | 184.917    | 0.2       | 309.083    | 0.7       |
| 66.083     | 7.3       | 190.917    | 8.0       | 315.333    | 7.6       |
| 72.417     | 1.1       | 197.00     | 0.5       | 321.500    | 1.0       |
| 78.583     | 7.7       | 203.333    | 8.5       | 327.750    | 7.4       |
| 84.833     | 1.0       | 209.667    | 0.1       | 334.167    | 1.0       |
| 91.083     | 7.4       | 215.750    | 8.1       | 340.250    | 7.4       |
| 97.417     | 1.1       | 222.083    | 0.4       | 346.500    | 1.3       |
| 103.583    | 7.9       | 228.167    | 8.5       | 353.000    | 7.1       |
| 109.915    | 0.8       | 234.667    | 0.1       | 359.167    | 1.3       |
| 116.167    | 7.5       | 240.750    | 8.0       |            |           |
| 122.333    | 0.9       | 246.915    | 0.5       |            |           |

Table 5-2 Tide Observations.

| <u>Symbol</u> | <u>Frequency (deg./hr)</u> |
|---------------|----------------------------|
| $O_1$         | 13.943036                  |
| $K_1$         | 15.041069                  |
| $P_1$         | 14.958931                  |
| $K_2$         | 30.082137                  |
| $N_2$         | 28.439730                  |

### 5.1 Computations and the Results

The computations have been completed in three steps. First, least squares approximations were done to determine the coefficients of the polynomials that will predict the range ratio ( $r_i$ ) and the time difference (correction to time) at a point  $P_i$  ( $\phi_i, \lambda_i$ ). Second, a least squares polynomial approximation of the observed time series at the reference station (Table 5-2) was completed to determine the coefficients of the polynomial that will predict the height of tide  $\bar{h}_0(t)$  at the reference station at any time  $t \in M$ . Finally, using the results of the first two steps, the observed geodetic coordinates at a point  $P_i(\phi_i, \lambda_i)$  and the observed time at the location, the height of tide at the ship was computed for the determination of the reduced depth.

#### 5.1.1 Determination of the Coefficients of the Approximating Polynomials

Of the 35 secondary gauge stations spread around the Bay of Fundy, 21 of them that are located around the main body of the Bay were used. Because of the intervening

peninsula which bifurcates the Bay at about longitude  $64^{\circ} 55'$ , the tidal wave propagation have been greatly affected along the two branches. A single analytical cotidal model for the entire area could not therefore be produced. The Bay has been divided into three sections numbered I, II and III in Figure 5-1. We have used the 21 secondary stations to model section I (those stations marked with \* in Table 5-1 under remarks column). It should be noted that the origin of the local Cartesian coordinate system is approximately at the centre of the area being modelled ( $\phi_0 = 45^{\circ} 05' 00\text{N}$ ,  $\lambda_0 = 65^{\circ} 35' 00\text{W}$ ). The data at the reference station (Saint John) was not fixed giving a total of 22 data points for the approximation.

Using equation 4.11, it was deduced that the highest degree of polynomial possible with 22 data points is 3, giving a total of 16 coefficients and 6 degrees of freedom. This does not however mean that the polynomial of degree 3 will give a better approximation than polynomials of degree 1 or 2. In Table 5-3 the degrees of the polynomials and their associated a posteriori variance factors are tabulated. Two of the functions (Range ratio and Function A) have their variance factors reach a minimum at degree 2, while the variance factor of the third function (Function B) varies more slowly at degree 2. The conclusion is that the polynomial of degree 2 will give the best approximation with this data.

The approximation for time lag required some extra

| n | Degree of Freedom (df) | Std. Dev. of Obs. $\sigma_L$ (m) | Range Ratio $\hat{\sigma}_0^2$ | Function A = R cos(v) $\hat{\sigma}_0^2$ | Function B = R sin(v) $\hat{\sigma}_0^2$ |
|---|------------------------|----------------------------------|--------------------------------|--|--|
| 1 | 18                     | 0.1                              | 0.94497                        | 14.02610                                 | 20.60978                                 |
| 2 | 13                     | 0.1                              | 0.84280                        | 12.40109                                 | 13.66546                                 |
| 3 | 6                      | 0.1                              | 1.14585                        | 17.50086                                 | 13.43903                                 |

Table 5-3 A Posteriori Variance Factor for Various Degrees of the Polynomials.

data manipulation. First, the time differences given in minutes were converted to angular measure using the relation

$$12 \text{ hrs} = 360^\circ,$$

$$1 \text{ hr} = 30^\circ,$$

$$1 \text{ min} = 0.5''.$$

(the Bay of Fundy tide is mainly semi-diurnal). Attempts to approximate the time lag converted to angular measure yielded large variance factors which of course decreased with increase in the degree of the polynomial. Unfortunately the highest degree of polynomial with the data available is 3. The conclusion reached was that the time lag distribution is not simple enough to be approximated by lower degrees of the polynomials.

From Chapter II

$$\begin{aligned} h(t) &= \frac{1}{2} R \cos(\omega_k t + \alpha_k) , \\ &= \frac{1}{2} R \cos \alpha_k \cos \omega_k t + \frac{1}{2} R \sin \alpha_k \sin \omega_k t, \\ &= A \cos \omega_k t + B \sin \omega_k t, \end{aligned} \quad (5.1)$$

where  $A = \frac{1}{2} R \cos \alpha_k$  . (5.2)

$$B = \frac{1}{2} R \sin \alpha_k . \quad (5.3)$$

$\alpha_k$  is time lag (or phase lag), and  $R$  is the mean tide range at the station.

Also

$$\alpha_k = \text{Arctan} (B/A) , \quad (5.4)$$

$$\frac{1}{2}R = (A^2 + B^2)^{1/2} . \quad (5.5)$$

A and B can therefore be evaluated at each station using equations 5.2 and 5.3 respectively. We can now seek for the polynomials that can predict A and B at any point  $P_i$  ( $\phi_i, \lambda_i$ ). Once A and B are predicted, the predicted time lag (phase lag) can be obtained using equation 5.4. The associated variance (assuming no correlation between A and B eg.  $\sigma_{AB} = 0$ ) is given by

$$\sigma_{\alpha}^2 = \left( \frac{\partial \alpha}{\partial A} \right)^2 \sigma_A^2 + \left( \frac{\partial \alpha}{\partial B} \right)^2 \sigma_B^2 , \quad (5.6)$$

where

$$\frac{\partial \alpha}{\partial A} = \frac{1}{1 + (B/A)^2} \times (-B/A^2) = \frac{B}{A^2(1 + (B/A)^2)} , \quad (5.7)$$

$$\frac{\partial \alpha}{\partial B} = \frac{1}{1 + (B/A)^2} \times \frac{1}{A} = \frac{1}{A(1 + (B/A)^2)} . \quad (5.8)$$

$\sigma_A^2$  and  $\sigma_B^2$  are prediction variances of A and B respectively from the least squares approximations. For weighting, it was assumed that all the stations have been observed independently with equal amount of care. The standard error of the observed range was set at 0.1m.

If observed data is used, it is pertinent to note the following:

- (i) The standard error of the observed mean range should be computed from the observed data using the relation

$$\sigma_R = \sqrt{\sum (R_{mD} - \bar{R}_m)^2 / n} . \quad (5.9)$$

where  $R_{mD}$  is the daily mean range,  $\bar{R}_m$  is the mean of mean ranges,  $n$  is the number of observations.

- (ii) A and B should be computed from equation 5.1 in the least squares sense using observed heights and the dominant constituent frequency in the semi-diurnal or diurnal band, depending on the type of tide.

In Table 5-4, the Fourier coefficients and their associated standard deviations for the range ratio and time lag are tabulated. The last four Fourier coefficients in the range ratio and function A have been eliminated, and in function B, two of the Fourier coefficients have been eliminated in the middle. In Table 5-5, the original coefficients of the polynomials are tabulated. Because of the discarding of the last four Fourier coefficients in the range ratio and function A, only five original coefficients can be recovered. In function B where the Fourier coefficients discarded are not the last ones, all the 9 original coefficients were recovered. (Note, each Fourier coefficient was tested against its standard deviation)

To compare the analytical cotidal model with other cotidal charts, the area was divided into a rectangular grid of 10' latitude and 10' longitude (Figure 5-2), and the values of range ratios ( $r$ ) and time lags have been

| R A N G E R A T I O     |                | T I M E L A G           |                |                         |                |
|-------------------------|----------------|-------------------------|----------------|-------------------------|----------------|
| Coeff.(C <sub>r</sub> ) | $\sigma_{C_r}$ | Coeff.(C <sub>A</sub> ) | $\sigma_{C_A}$ | Coeff.(C <sub>B</sub> ) | $\sigma_{C_B}$ |
| 1.049                   | 0.01957        | 3.970                   | 0.07508        | 0                       | 0              |
| 0.4623E-5               | 0.330E-6       | 0.1783E-4               | 0.1265E-5      | 0.4906E-5               | 0.1237E-5      |
| 0.1940E-10              | .6191E-11      | 0.7554E-10              | 0.2375E-10     | 0.7091E-10              | 0.2321E-10     |
| 0.2126E-5               | 0.5570E-6      | 0.8618E-5               | 0.2138E-5      | 0.3977E-5               | 0.2089E-5      |
| -0.2648E-10             | 0.1350E-10     | -0.1140E-9              | 0.5178E-10     | 0                       | 0              |
| 0                       | 0              | 0                       | 0              | 0.1728E-14              | 0.1004E-14     |
| 0                       | 0              | 0                       | 0              | 0.2869E-9               | 0.1042E-9      |
| 0                       | 0              | 0                       | 0              | 0.2024E-14              | 0.1582E-14     |
| 0                       | 0              | 0                       | 0              | -0.5218E-19             | 0.3350E-19     |

Table 5-4 Fourier Coefficients After Discarding those of them greater than their Standard Deviations.



| R A N G E R A T I O     |                | T I M E L A G           |                |                         |                |
|-------------------------|----------------|-------------------------|----------------|-------------------------|----------------|
| Coeff.(C <sub>r</sub> ) | $\sigma_{C_r}$ | Coeff.(C <sub>A</sub> ) | $\sigma_{C_A}$ | Coeff.(C <sub>B</sub> ) | $\sigma_{C_B}$ |
| 1.120                   | 0.03537        | 4.264                   | 0.1357         | -0.5691                 | 0.16978        |
| 0.4041E-5               | 0.5680E-6      | 0.1550E-4               | 0.2179E-5      | 0.8730E-5               | 0.3614E-5      |
| 0.1364E-10              | 0.7644E-11     | 0.5374E-10              | 0.2932E-10     | 0.1484E-9               | 0.4318E-10     |
| 0.1247E-5               | 0.7150E-6      | 0.4835E-5               | 0.2742E-5      | 0.1695E-4               | 0.5526E-5      |
| -0.2648E-10             | 0.1350E-10     | -0.1140E-9              | 0.5177E-10     | -0.2617E-9              | 0.1938E-9      |
| 0                       | 0              | 0                       | 0              | -0.3887E-14             | 0.2208E-14     |
| 0                       | 0              | 0                       | 0              | 0.4269E-9               | 0.1391E-9      |
| 0                       | 0              | 0                       | 0              | 0.5786E-15              | 0.1834E-14     |
| 0                       | 0              | 0                       | 0              | -0.5218E-19             | 0.3349E-19     |

Table 5-5 The Original Coefficient of the Polynomials and their Associated Standard Deviations.

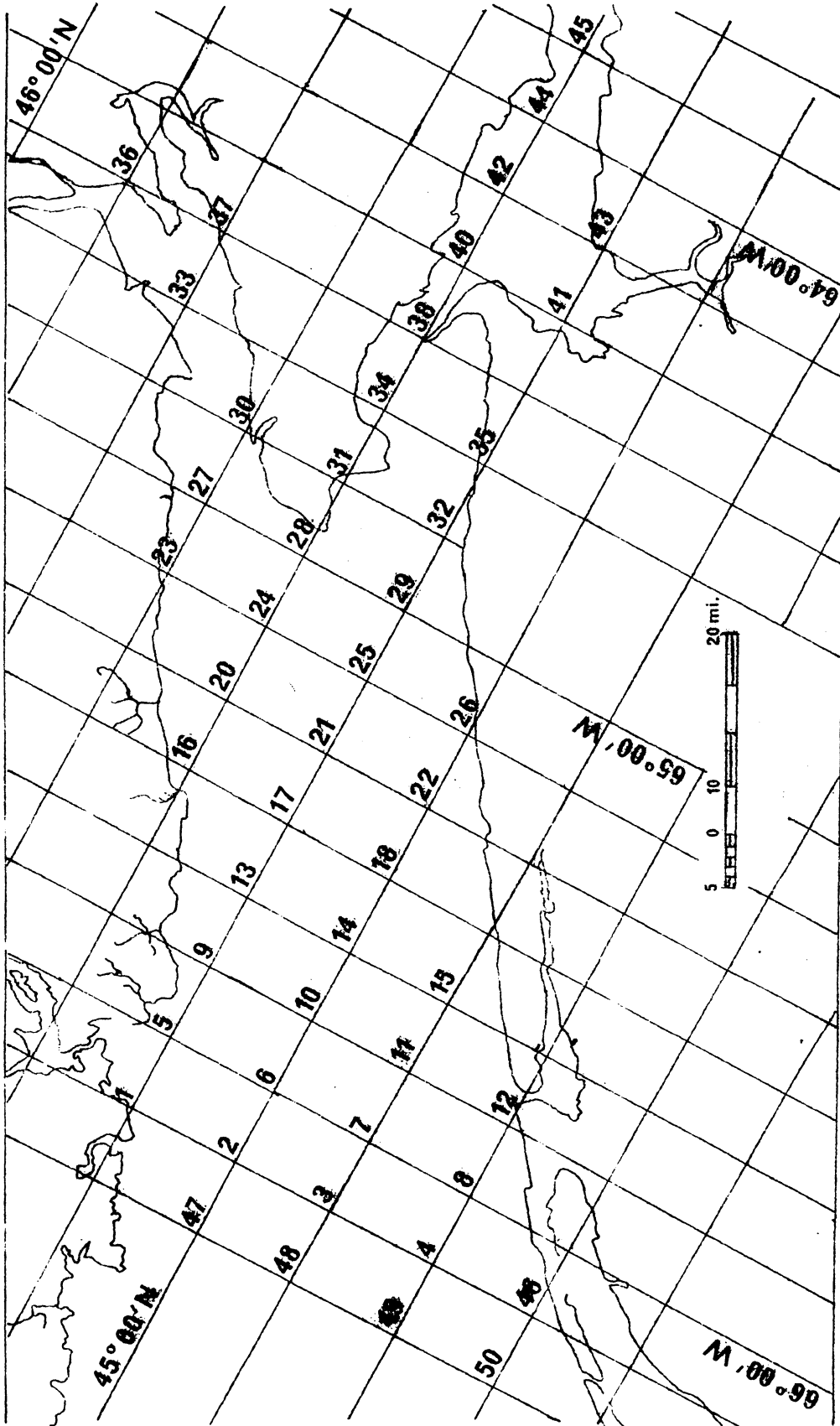


Figure 5-2

Grid Numbering

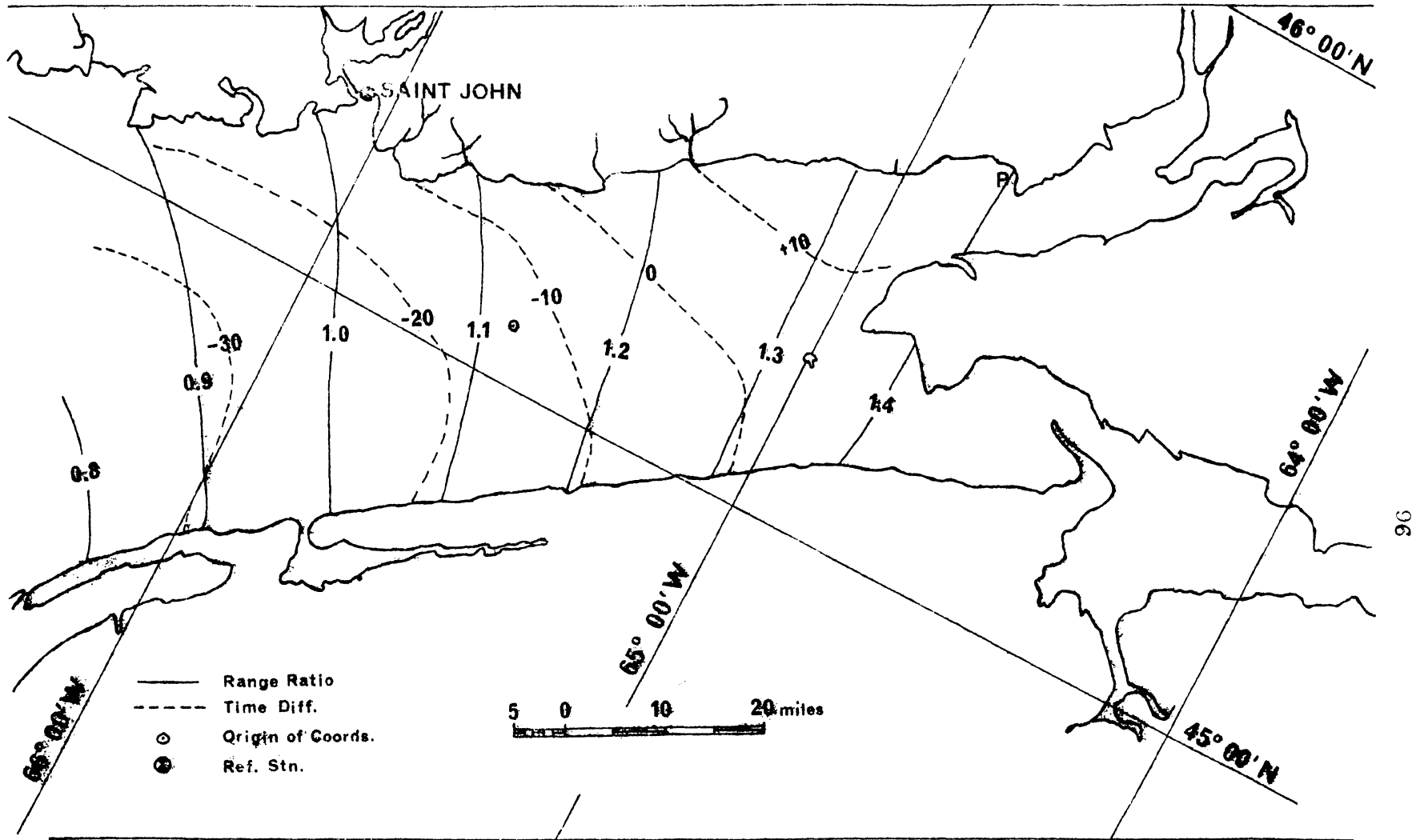


Figure 5-3

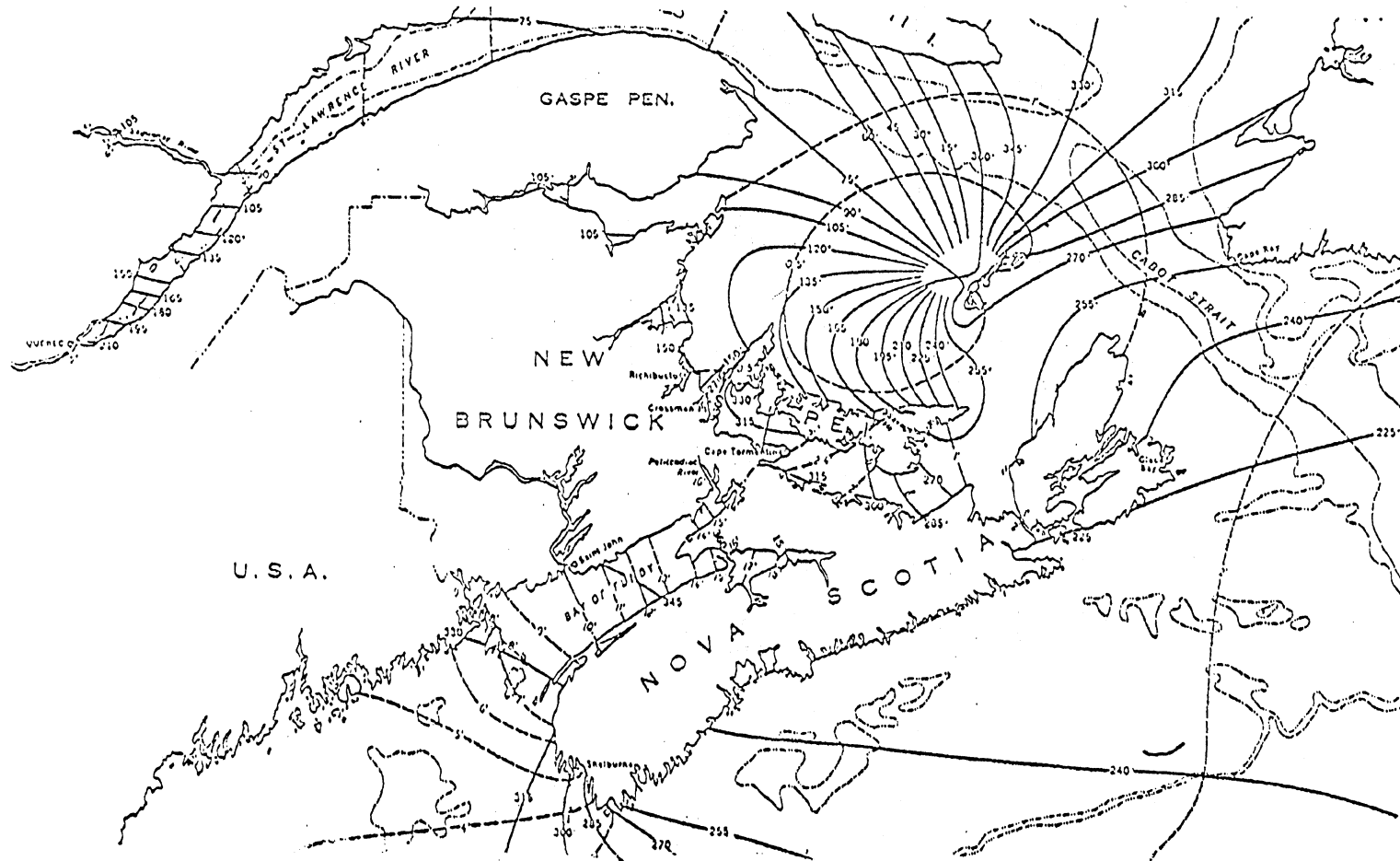
Bay of Fundy -- Range/Time Co-Tidal Curve from the Analytical Co-Tidal Models

PREDICTED RANGE RATIOS AND TIME LAGS AT THE GRID POINTS

| NO | LATITUDE  | LONGITUDE  | RANGE RATIO | SIGMA      | TIME LAG   | SIGMA      |
|----|-----------|------------|-------------|------------|------------|------------|
| 1  | 45.166670 | -66.166670 | 0.986543    | 0.4405D-01 | -13.338520 | 0.56220 01 |
| 2  | 45.000000 | -66.166670 | 0.940871    | 0.3616D-01 | -26.008451 | 0.78810 01 |
| 3  | 44.833330 | -66.166670 | 0.895199    | 0.3015D-01 | -33.234233 | 0.91950 01 |
| 4  | 44.666670 | -66.166670 | 0.849530    | 0.2727D-01 | -34.578871 | 0.92830 01 |
| 5  | 45.166670 | -66.000000 | 1.022218    | 0.4303D-01 | -13.691588 | 0.53190 01 |
| 6  | 45.000000 | -66.000000 | 0.982984    | 0.3624D-01 | -26.041995 | 0.72620 01 |
| 7  | 44.833330 | -66.000000 | 0.943749    | 0.3145D-01 | -31.769547 | 0.83120 01 |
| 8  | 44.666670 | -66.000000 | 0.904517    | 0.2664D-01 | -30.424472 | 0.78250 01 |
| 9  | 45.166670 | -65.833330 | 1.062573    | 0.4136D-01 | -12.923846 | 0.43790 01 |
| 10 | 45.000000 | -65.833330 | 1.029776    | 0.3590D-01 | -24.231532 | 0.64620 01 |
| 11 | 44.833330 | -65.833330 | 0.996980    | 0.3323D-01 | -28.197832 | 0.69630 01 |
| 12 | 44.666670 | -65.833330 | 0.964185    | 0.3401D-01 | -24.349502 | 0.69790 01 |
| 13 | 45.166670 | -65.666670 | 1.107604    | 0.3872D-01 | -11.163316 | 0.42950 01 |
| 14 | 45.000000 | -65.666670 | 1.081246    | 0.3519D-01 | -20.811055 | 0.54230 01 |
| 15 | 44.833330 | -65.666670 | 1.054887    | 0.3578D-01 | -22.808907 | 0.55810 01 |
| 16 | 45.166670 | -65.500000 | 1.177242    | 0.4110D-01 | 0.276973   | 0.49900 01 |
| 17 | 45.000000 | -65.500000 | 1.157322    | 0.3525D-01 | -8.537109  | 0.38310 01 |
| 18 | 44.833330 | -65.500000 | 1.137400    | 0.3475D-01 | -15.998606 | 0.42910 01 |
| 19 | 44.666670 | -65.500000 | 1.117477    | 0.3980D-01 | -15.874801 | 0.51170 01 |
| 20 | 45.166670 | -65.333330 | 1.225201    | 0.3593D-01 | 0.641768   | 0.44460 01 |

|    |           |            |          |            |            |            |
|----|-----------|------------|----------|------------|------------|------------|
| 21 | 45.100070 | -65.333330 | 1.211718 | 0.31890-01 | -5.178430  | 0.32270 01 |
| 22 | 45.000000 | -65.333330 | 1.198233 | 0.35810-01 | -10.012239 | 0.30280 01 |
| 23 | 45.800000 | -65.166670 | 1.284885 | 0.42100-01 | 21.128669  | 0.39560 01 |
| 24 | 45.333330 | -65.166670 | 1.277837 | 0.30870-01 | 6.951205   | 0.36930 01 |
| 25 | 45.100070 | -65.166670 | 1.270790 | 0.29870-01 | -1.217189  | 0.27330 01 |
| 26 | 45.000000 | -65.166670 | 1.263743 | 0.39890-01 | -3.080484  | 0.41980 01 |
| 27 | 45.500000 | -65.000000 | 1.335766 | 0.39320-01 | 16.493540  | 0.35030 01 |
| 28 | 45.333330 | -65.000000 | 1.335157 | 0.28310-01 | 7.208467   | 0.29400 01 |
| 29 | 45.100070 | -65.000000 | 1.334547 | 0.32470-01 | 3.218478   | 0.26300 01 |
| 30 | 45.500000 | -64.833330 | 1.391329 | 0.40120-01 | 10.978109  | 0.26340 01 |
| 31 | 45.333330 | -64.833330 | 1.397156 | 0.31700-01 | 7.417273   | 0.27780 01 |
| 32 | 45.100070 | -64.833330 | 1.402984 | 0.40970-01 | 8.003351   | 0.31300 01 |
| 33 | 44.800000 | -65.166670 | 0.803856 | 0.23510-01 | -29.008920 | 0.78010 01 |
| 34 | 45.000000 | -65.333330 | 0.903441 | 0.36180-01 | -23.869221 | 0.77070 01 |
| 35 | 44.833330 | -65.333330 | 0.851332 | 0.29850-01 | -32.240970 | 0.94920 01 |
| 36 | 44.666670 | -65.333330 | 0.799226 | 0.27320-01 | -36.402650 | 0.97790 01 |
| 37 | 44.500000 | -65.333330 | 0.747117 | 0.30190-01 | -39.869593 | 0.81100 01 |

Table 5-6 Predicted Values at Grid Intersections



Amplitude of the Main Semi-Diurnal Tide in Feet - - - 2' - - - -

Phase Lag of the Main Semi-Diurnal Tide - - 150° - - - -

(30° = Approx. 1 hr. in time)

Figure 5-4

The Average Progression of Semi-Diurnal

Tide in the Bay of Fundy (Dohler, 1966).

predicted at each grid intersection. The co-range curves and the co-time curves were plotted as shown in Figure 5-3. Figure 5-2 shows the grid numbering, and in Table 5-6, the predicted values at each grid intersection and their associated standard deviations are tabulated. The cotidal curves from the proposed analytical models compared favourably with the cotidal curves (Figure 5-4) taken from 'Tides in Canadian Waters' [Dohler, 1966] showing the progression of semi-diurnal tides in the Bay of Fundy.

#### 5.1.2 Least Squares Polynomial Approximation of Observed Time Series at the Reference Station

In this case, the heights of the tide defined at discrete times ( $t_i$ ) in the time interval  $M$  are given and it is required to determine the coefficients of the polynomial that will best predict the height of tide  $h(t)$  at any other time  $t \in M$ . A one dimensional trigonometric polynomial (Eqn. 2.30, Chapter II, Section 2.2) and the 7 constituent frequencies listed on page 82 have been used. The number of coefficients is given by

$$U = 2 N_{con} + 1 \quad , \quad (5.10)$$

where  $N_{con}$  is the number of constituent frequencies being used. For weighting, it was assumed that each height was observed independently, with equal amount of care and precision, and  $\sigma_{h(t)} = 0.05$  m. The weight matrix is therefore

$$P_{mxm} = \text{Diag} \left( \frac{1}{\sigma_{h_1}^2} \quad \frac{1}{\sigma_{h_2}^2} \quad \dots \quad \frac{1}{\sigma_{h_m}^2} \right) \quad . \quad (5.11)$$

In Table 5-7, the Fourier coefficients and the recovered coefficients of the approximating polynomial of the observed time series, and their associated standard deviations are tabulated. Two Fourier coefficients were discarded in the middle of the series thus all the 15 original coefficients were recovered.

### 5.1.3 Tidal Reduction

For this set of computations, simulated sounding observations (corresponding in location to the 22 data points and with all observations made within the time interval M) were used to illustrate a proposed reduction algorithm. At each sounding location  $i$ , the depth ( $D_i$ ), the time ( $t$ ) and the geodetic coordinates ( $\phi_i, \lambda_i$ ) or the local Cartesian coordinates ( $x_i, y_i$ ) are observed.

The arguments of the approximating polynomials for range ratios and time lags are the local Cartesian coordinates ( $x, y$ ) and the argument of the approximating polynomial for the heights of tide at the reference station is the time ( $t$ ). With the polynomial coefficients and their associated standard deviations stored in the computer, only the arguments ( $x_i, y_i$ ) are needed to predict the range ratio ( $r_i$ ) and the time lag ( $t_{c_i}$ ). The time lag is, in a sense, the correction to be applied to the observed time at the ship (sounding location  $i$ ) to get the equivalent time at the reference station. With the equivalent time at the reference station computed, the height of the tide at the reference station is predicted



| FOURIER COEFFS. AFTER TEST<br>AGAINST THEIR STD. DEVS. |                         | COEFFICIENTS OF THE ORIGINAL<br>POLYNOMIAL |                         |
|--|-------------------------|--|-------------------------|
| Coeff. ( $F_c$ )                                       | Std. Dev.<br>$\sigma_c$ | Coeff. (C)                                 | Std. Dev.<br>$\sigma_c$ |
| 4.279  | 0.5861E-2               | 4.279                                      | 0.6227E-2               |
| -5.339   | 0.9072E-2               | -1.951                                     | 0.1423                  |
| 2.335  | 0.02048                 | 2.226                                      | 0.1081                  |
| 0.4590   | 0.9760E-2               | -0.1756                                    | 0.2113                  |
| -0.0496  | 0.8784E-2               | 0.2592                                     | 0.3972                  |
| 0.0360   | 0.8436E-2               | 0.0678                                     | 0.0100                  |
| -0.0909  | 0.8295E-2               | -0.1031                                    | 0.8507E-2               |
| -0.1338  | 0.8309E-2               | -0.2305                                    | 0.0319                  |
| 0.1584   | 0.8360E-2               | 0.0335                                     | 0.0322                  |
| 0  | 0                       | 0.1392                                     | 0.0382                  |
| 0  | 0                       | 0.0832                                     | 0.0229                  |
| -0.4193  | 0.0954                  | 0.4132                                     | 0.1602                  |
| 0.9868   | 0.1508                  | -0.0302                                    | 0.3586                  |
| 0.4479   | 0.0840                  | 0.7741                                     | 0.1272                  |
| 0.4300   | 0.1259                  | 0.4300                                     | 0.1259                  |

Table 5-7 Coefficients of the Polynomial for the Observed Time Series at the Reference Station.

using the known time as the argument of the predicting polynomial. The height of tide at the ship, which is the required reduction, is obtained using equation 4.43. The reduced sounding is computed using equation 4.44. Applying the law of propagation of errors, the standard deviation of reduced sounding is given by

$$\sigma_{d_i} = \left[ \left( \frac{\partial d_i}{\partial D_i} \right)^2 \sigma_{D_i}^2 + \left( \frac{\partial d_i}{\partial h_i(t)} \right)^2 \sigma_{h_i(t)}^2 \right]^{1/2}, \quad (5.12)$$

where  $\sigma_{D_i}$  is the standard deviation of the depth sounded,  $\sigma_{h_i(t)}$  is the standard deviation of the predicted height at the ship,  $\frac{\partial d_i}{\partial D_i} = 1$  and  $\frac{\partial d_i}{\partial h_i(t)} = 1$ . The heights of the tide at the ship, required to reduce the soundings, are tabulated in Table 5-8 along with their estimated standard deviations. The predicted time lags and range ratios are compared with the original data set (observed values) as shown in Table 5-9. At this stage it is important to mention that normality in the distribution of residuals was assumed and chi square tests on the variance factor performed at 95% confidence level. The test passed for the range ratio and observed time series approximations but failed for the time lag. There are several possible reasons for the failure of this test and as such a definite conclusion cannot be made without performing several other statistical tests [Vaniček and Krakiwsky, Chapter 13, 1978]. However, it can be concluded from our earlier discussions (Section 5.1) that either the time lag is not simple enough to be approximated by the lower order polynomial or the information available is

not sufficient to approximate the time lag. In the present circumstance it may be safer to assume  $\sigma_0^2$  known and equal to 1, so that

$$\sum \hat{c} = \sigma_0^2 N^{-1}. \quad (5.13)$$

| TIDAL REDUCTIONS |           |            |            |              |             |             |           |              |            |
|------------------|-----------|------------|------------|--------------|-------------|-------------|-----------|--------------|------------|
| NOM              | LATITUDE  | LONGITUDE  | OPS. DEPTH | TIME AT SHIP | TIME AT REF | TIDE AT REF | PR. RATIO | TIDE AT SHIP | STDEV      |
| 1                | 45.350000 | -04.810007 | 12.000     | 2.750        | 2.024       | 6.855       | 1.403     | 9.616        | 0.89760 00 |
| 2                | 45.300000 | -04.783333 | 13.020     | 3.150        | 3.124       | 7.181       | 1.418     | 10.183       | 0.92080 00 |
| 3                | 45.233333 | -04.516667 | 14.280     | 4.500        | 4.274       | 7.218       | 1.538     | 11.105       | 0.95730 00 |
| 4                | 45.250000 | -05.000000 | 13.720     | 4.833        | 4.758       | 6.935       | 1.335     | 9.255        | 0.66780 00 |
| 5                | 45.000000 | -05.000007 | 13.820     | 5.117        | 5.403       | 6.314       | 1.306     | 8.246        | 0.54260 00 |
| 6                | 44.800000 | -05.533333 | 10.000     | 6.500        | 6.775       | 4.394       | 1.100     | 4.834        | 0.48400 00 |
| 7                | 44.400000 | -05.833333 | 13.940     | 25.133       | 25.312      | 4.035       | 0.911     | 3.677        | 0.49290 00 |
| 8                | 44.500000 | -06.100000 | 10.120     | 28.633       | 28.905      | 7.391       | 0.823     | 6.110        | 0.63200 00 |
| 9                | 44.403333 | -06.083333 | 12.060     | 29.250       | 29.647      | 7.026       | 0.829     | 5.828        | 0.58630 00 |
| 10               | 44.200007 | -06.350000 | 9.880      | 30.300       | 30.737      | 5.819       | 0.668     | 3.865        | 0.70690 00 |
| 11               | 44.200000 | -06.400000 | 9.500      | 31.500       | 32.020      | 3.871       | 0.643     | 2.491        | 0.57440 00 |
| 12               | 44.333333 | -06.116667 | 8.520      | 32.250       | 32.425      | 3.253       | 0.773     | 2.539        | 0.51050 00 |
| 13               | 44.200000 | -06.166667 | 7.080      | 32.833       | 32.755      | 2.762       | 0.721     | 2.007        | 0.49390 10 |
| 14               | 44.000000 | -06.800000 | 8.750      | 33.750       | 34.285      | 1.315       | 0.650     | 0.861        | 0.35240 00 |
| 15               | 44.003333 | -06.550000 | 3.350      | 35.167       | 35.111      | 1.205       | 0.752     | 0.906        | 0.31720 00 |
| 16               | 45.000007 | -07.050000 | 12.320     | 45.333       | 45.050      | 2.882       | 0.827     | 2.363        | 0.59010 00 |
| 17               | 45.233333 | -06.050000 | 13.100     | 46.583       | 46.700      | 1.294       | 1.027     | 1.329        | 0.43880 00 |
| 18               | 45.300000 | -05.533333 | 14.230     | 145.583      | 145.448     | 2.118       | 1.170     | 2.478        | 0.36130 00 |
| 19               | 45.500007 | -04.966667 | 15.130     | 149.300      | 148.966     | 2.017       | 1.346     | 2.714        | 0.53880 00 |
| 20               | 45.600000 | -04.783333 | 16.800     | 152.300      | 152.099     | 7.340       | 1.404     | 10.305       | 0.98440 00 |
| 21               | 45.400007 | -04.850000 | 15.850     | 155.917      | 155.744     | 6.132       | 1.386     | 8.501        | 0.63400 00 |
| 22               | 45.200007 | -06.066667 | 13.690     | 156.250      | 156.300     | 5.093       | 1.032     | 5.257        | 0.42410 00 |

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Table 5.8 Tidal Reductions

| Station Index No  | Obs. Time Lag | Predicted Time Lag | Diff. | Obs. Range Ratio | Predicted Range Ratio | Diff.  |
|---|---------------|--------------------|-------|------------------|-----------------------|--------|
| * 0065(22)  | 0             | -3                 | +3    | 1.0              | 1.032                 | -0.032 |
| 0001(14)  | -28           | -32                | +4    | 0.661            | 0.655                 | +0.006 |
| 0015(15)  | +5            | +3                 | +2    | 0.673            | 0.752                 | -0.079 |
| 0040(16)  | +16           | +16                | 0     | 0.900            | 0.827                 | +0.073 |
| 0060(17)  | -10           | -7                 | -3    | 0.996            | 1.027                 | -0.031 |
| 0129(18)  | +9            | +8                 | +1    | 1.201            | 1.170                 | +0.031 |
| 0140(19)  | +19           | +20                | -1    | 1.325            | 1.346                 | -0.021 |
| 0150(20)  | +17           | +12                | +5    | 1.410            | 1.404                 | 0.006  |
| 0225(21)  | +11           | +10                | +1    | 1.317            | 1.386                 | -0.069 |
| 0235(1)   | -1.0          | +8                 | -9    | 1.311            | 1.403                 | -0.092 |
| 0240(2)   | +16           | +8                 | +8    | 1.456            | 1.418                 | +0.038 |
| 0305(3)   | +12           | +14                | -2    | 1.490            | 1.538                 | -0.048 |
| 0312(4)   | 0             | +5                 | -5    | 1.361            | 1.335                 | +0.026 |
| 0315(5)   | -12           | +1                 | -13   | 1.265            | 1.306                 | -0.041 |
| 0320(6)   | -14           | -16                | +2    | 1.060            | 1.100                 | -0.040 |
| 0330(7)   | -16           | +1                 | -17   | 0.956            | 0.911                 | +0.045 |
| 0335(8)   | -18           | -26                | +8    | 0.843            | 0.828                 | +0.015 |
| 0336(9)   | -37           | -24                | -13   | 0.761            | 0.829                 | -0.068 |
| 0340(10)  | -34           | -26                | -8    | 0.721            | 0.668                 | +0.053 |
| 0345(11)  | -34           | -31                | -3    | 0.713            | 0.643                 | +0.070 |
| 0353(12)  | +18           | -10                | +28   | 0.721            | 0.778                 | -0.057 |
| 0355(13)  | 18            | +5                 | +13   | 0.673            | 0.721                 | -0.048 |
| <u>Summary:</u>   |               |                    |       |                  |                       |        |
| Time Lag : $0 < \text{Diff.} <  28 $                        |               |                    |       |                  |                       |        |
| Range Ratio : $ 0.005  < \text{Diff.} <  0.1 $              |               |                    |       |                  |                       |        |
| Time Lag, RMS of the Diff. (observed-predicted) = 9.44 min  |               |                    |       |                  |                       |        |
| Range Ratio, RMS of the Diff. (observed-Predicted) = 0.0505 |               |                    |       |                  |                       |        |

Table 5-9 Difference Between Predicted and Observed Values.

\* Numbers in brackets corresponds to the serial numbers in Table 5-8.

## VI CONCLUSIONS AND RECOMMENDATIONS

The objective of this work has been to produce analytical cotidal models, using observed data or existing cotidal charts, which could be stored conveniently in a computer so that when observed sounding data are input, the output would be reduced soundings. The principal advantages of the proposed analytical scheme are the following.

- (i) The analytical models can be obtained and updated using the observed data in addition to that of already produced charts. This allows up-dating the model when more observations are available.
- (ii) This scheme does not require large computer storage space. For example, instead of storing many digitized numbers, the digitized values are used to determine a few coefficients of the best approximating polynomials.
- (iii) These models allow for the rigorous propagation of errors. With associated estimated standard deviations, the reliability of the final result can be easily obtained.
- (iv) A degree of flexibility is offered. It is convenient to use data from existing cotidal charts, observations or a combination of the two.

Least squares polynomial approximation is applied to either

- (i) recover a function  $F(x)$  from a known set of its values, or
- (ii) to replace the known function in further computations by a more trackable polynomial.

The problem of least squares polynomial approximation as applied in this work is that defined by (i) above. It would be interesting to view the problem as in (ii) above and apply it to the Laplace tidal equations to obtain the necessary polynomials.

From the test computations using the data on the Bay of Fundy, the computer effective run time is 28.46 secs and the storage space is 336,136 Bytes for Least squares polynomial approximation for range ratios and time lags. For the polynomial approximation for time series at the reference station the effective run time is 23.03 secs and the storage space is 299,288 Bytes. For the Tidal Reduction therefore we have a total of 42 coefficients and their associated standard deviations to store in the computer. In the program to execute this for 22 sounding stations, the time of execution was 0.76 sec and the storage space used was 14,480 Bytes. The result also shows that the water level at a location  $(\phi_i, \lambda_i)$  can be predicted with a standard deviation  $(\sigma_{h_i})$  of 0.5 m or better.

It is recommended that the prediction of tides at the reference station with the polynomial should be done

strictly within the time interval  $M$  used in the least squares approximation of the observed series. When extrapolation is required, it is advisable to use the amplitude/phase analytical cotidal models and carry out the prediction using the procedure described in Chapter II, Section 2.2.

Finally, since the data immediately available was not adequate to fully test the proposed analytical schemes, it is suggested that proper data be obtained to facilitate complete testing.



## REFERENCES

- 'Admiralty Manual of Hydrographic Surveying,' Vol. 2 (1969)  
Hydrographer of the Navy, Taunton, Somerset.
- Atlantic Tidal Power Engineering and Management Committee,  
(1969), 'Report on the Feasibility of Tidal Power  
Development in the Bay of Fundy, Appendix 1,' Tides  
and Tidal Regime.
- Balogun, A.A.: (1977), 'Parameterization of Systematic Errors  
in Terrestrial Geodetic Networks,' M.Sc.E Thesis,  
University of New Brunswick, Fredericton.
- Bye, J.A.T and R.A. Heath.: (1975), 'The New Zealand Semi-  
diurnal Tides,' Journal of Marine Research, Vol. 33.
- 'Canadian Tide And Current Tables,' Vol. I, (1978), Canadian  
Hydrographic Service, Ottawa.
- Cartwright, D., W. Munk, and B. Zetler.: (1969), 'Pelagic  
Tidal Measurements,' National Institute of Oceano-  
graphy, U.K.
- Christodoulidis D.: (1973), 'Determination of Vertical Crustal  
Movements From Scattered Relevelings,' M.Sc.E Thesis,  
University of New Brunswick, Fredericton.
- DeWolfe, D.L.: (1977), 'Tidal Measurement Program of the Bay  
of Fundy-Gulf of Maine Tidal Regime,' Light House  
#15, The Journal of the Canadian Hydrographer's  
Association.
- Dohler, G.: (1966), 'Tides in Canadian Waters,' Canadian  
Hydrographic Services, Marine Science Branch,  
Department of Mines and Technical Surveys, Ottawa.
- Dohler, G.C. and L.F. Ku.: (1969), 'Presentation and Assess-  
ment of Tides and Water Level records for Geophysical  
Investigations,' Symposium on Recent Crustal Move-  
ments, Ottawa.
- Dronkers, J.J. and J.C. Schonfeld.: (1954), 'Tidal Computa-  
tions in Shallow Water,' Research Division,  
Rijkswaterstaat, the Netherlands.
- Dronkers, J.J.: (1960), 'Tidal Computations in Coastal Areas,'  
Delta Project, A symposium; American Society of  
Civil Engineers, New York.

- Dronkers, J.J.: (1964). 'Tidal Computations in Rivers and Coastal Waters,' North-Holland Publishing Company, Amsterdam.
- Dronkers, J.J.: (1970), 'The Morphological Changes in the neighbourhood of Rotterdam Europort and the New Approach Channel,' Paper presented at the Centenary Conference on Ports and Harbour Managements, Calcutta.
- Dronkers, J.J.: (1970), 'Research for the Coastal Area of the Delta Region of the Netherlands,' Coastal Engineering Vol. III, pp. 1783-1801.
- Dronkers, J.J.:(1972), 'Tidal Theory and Computations,' Hydraulic Dept. of Delta Works, The Hague, the Netherlands.
- Freeman, N.G. and T.S. Murty, : (1976), 'Numerical Modelling of Tides in Hudson Bay,' Journal of Fisheries Research Board of Canada 33: 2345-2361.
- Gallagher, S. Brent and W.H. Munk, : (1971), 'Tides in Shallow Water and Spectroscopy,' Dept. of Oceanography, University of Hawaii, Honolulu.
- Garrett, C. and D. Greenberg, : (1976), 'Predicting Changes in Tidal Regime: The Open Boundary Problem,' Journal of Physical Oceanography, Vol. 7, March 1977.
- Garrett, C.J.R. and W.H. Munk. : (1971), 'The age of the tide and the Q of the ocean,' Deep-sea Research, Vol. 18, pp. 493-503, Pergaman Press, Great Britain.
- Godin, G: (1972), 'The Analysis of Tides,' University of Toronto Press, Toronto.
- Gordeev, R.G.; B.A. Kagan and E.V. Polyakov: (1976), 'The Effects of Loading and Self-Attraction on Global Ocean Tides: The Model and the result of a Numerical Experiment,' Journal of Physical Oceanography, Vol. 7, pp. 161-170.
- Hendershott, M. and W. Munk: (1970), 'Tides,' Scripps Institution of Oceanography, La Jolla, California.
- Hendershott, M.C. (1972), 'The Effects of Solid Earth Deformation on Global Ocean Tides', Journal of Geophysical Research, Vol. 29, pp. 389-402.
- Hendershott, M.C. (1972), 'Ocean Tides,' Paper presented at the 1st GEOP. Research Conference on Tides, Columbus.

- Krakiwsky, E.J. and D.E. Wells. (1971), 'Coordinate Systems in Geodesy'. Lecture Notes #16, University of New Brunswick, Fredericton.
- Krakiwsky, E.J.: (1975), 'A Synthesis of recent advances in the method of Least Squares,' Lecture notes #42, Dept. of Surveying Engineering, University of New Brunswick, Fredericton.
- Lambert, A.: (1974), 'Earth Tide Analysis and Prediction by the Response Method,' Journal of Geophysical Research, Vol. 7, #32, pp. 4952-4960.
- Larsen, J.C.: (1977), 'Cotidal Charts for the Pacific Ocean near Hawaii Using F. Plane Solution,' Journal of Physical Oceanography, Vol. 7, pp. 100-109.
- Luther S. Douglas and C. Wunsch,: (1974), 'Tidal Charts of the Central Pacific Ocean,' Journal of Physical Oceanography, Vol. 5, 1975.
- Mosetti, F. and Manca.: (1972), 'Some Methods of Tidal Analysis,' Inter. Hydro. Review, Vol. 49, Monaco.
- Munk, W.H.: (1968), 'Once Again - Tidal Friction,' Journal of Royal Astronomical Society Vol. 9, pp. 352-375, Sunfield and Day Ltd., Sussex.
- Munk, W.H. and D.E. Cartwright.: (1966), 'Tidal Spectroscopy and Prediction,' Philosophical Transactions of the Royal Society of London, Vol. 259, pp. 533-581, May 1966.
- Munk, W., Snodgrass F. and M. Wimbush.: (1969), 'Tides Offshore: Transition from California Coastal to Deep-Sea Waters,' Geophysical Fluid Dynamics, Vol. 1, pp. 161-235.
- Munk, W.H. and B.D. Zetler.: (1975), 'The Optimum Wiggleness of Tidal Admittances,' Journal of Marine Research, Supp. 33: pp. 1-13.
- Nassar M.N. and P. Vaniček.: (1975), 'Levelling and Gravity,' Technical Report No. 33, University of New Brunswick, Fredericton.
- 'Our Restless Tides,' (1977), U.S. Department of Commerce, National Oceanic And Atmospheric Administration, National Ocean Survey.
- Schwarz, H.R.: (1973), 'Numerical Analysis of Symmetric Matrices,' Prentice-Hall, Inc. New Jersey.

- Thomson, D.B.: (1974), 'Mean Sea Level,' Dept. of Surveying Engineering, University of New Brunswick, Fredericton.
- Tinney, B.: (1977), 'Automated Tidal Reduction,' Light House #15, Journal of the Canadian Hydrographers' Association.
- Vaniček, P.: (1970), 'Further Development and Properties of the Spectral Analysis by Least Squares,' Department of Surveying Engineering, University of New Brunswick, Fredericton.
- Vaniček, P.: (1973), 'The Earth Tides,' Lecture Note #36, Dept. of Surveying Engineering, University of New Brunswick, Fredericton.
- Vaniček, P.: (1974), 'Introduction to Adjustment Calculus,' Lecture Note #35, Dept. of Surveying Engineering, University of New Brunswick, Fredericton.
- Vaniček, P. and D.E. Wells,: (1972), 'The Least-Squares Approximation and Related Topics,' Lecture Note #22. Dept. of Surveying Engineering, University of New Brunswick, Fredericton.
- Vanicek, P. and E.J. Krakiwsky.: (in prep.), 'The Concept of Geodesy'.
- Von Arx, S.W.: (1962), 'An Introduction to Physical Oceanography,' Addison-Wesley Pub. Comp. Reading.
- Wells, D.E. and E.J. Krakiwsky.: (1971), 'The Method of Least Squares,' Lecture Note #18, Dept. of Surveying Engineering, University of New Brunswick, Fredericton.
- Wemelsfelder, P.J. (1970), 'Mean Sea Level as a fact and as an illusion,' Symposium on Coastal Geodesy, Munich.
- White, J.C.E. (Lt. Commander): (1971), 'Hydrographic and Tidal Information for Deep Draught Ships In a Tidal Estuary,' Paper presented at the XIII<sup>th</sup> Inter. Cong. of Surveyors, Wiesbaden.
- Zetler, B., D. Cartwright and W. Munk.: (1969), 'Tidal Constants Derived from Response Admittances', Sixth International Symposium on Earth Tides, Strasbourg.

Zetler, D.B. and R.A. Cumming: (1966), 'A Harmonic Method for predicting Shallow-Water Tides,' Journal of Marine Research, 25(1). pp. 103-113.

Zetler, B. et al: (1975), 'Mode Tides,' Journal of Physical Oceanography, Vol. 5, 430-441.

A P P E N D I C E S

## I OUTLINE OF THE LEAST SQUARES APPROXIMATION THEORY

Least squares polynomial approximation is applied to either

- (i) recover a function  $F(x)$  from a known set of its values, or
- (ii) replace the known function  $F(x)$  in further computations by a more trackable polynomial.

The problem of least squares polynomial approximation as used in this report is that defined by (i) above. A brief outline of the least squares approximation theory due mainly to Vaniček and Wells, [1972] is here given.

Given:

- (i) a function  $F$  defined on a finite set  $M$   
 $M \equiv \{X_1, X_2, \dots, X_m\}$ ,  $M$  discrete  
 $M \equiv [a, b]$ ,  $M$  compact
- (ii) a base  $\Psi = \psi_1, \psi_2, \dots, \psi_u$ , a set of  $u$  linearly independent prescribed functions from the functional space  $G_m$ ,
- (iii) a weight function  $W$ , defined and non-negative on  $M$ ,

then the problem of least squares approximation is to determine the vector of coefficient  $(C_1, C_2, \dots, C_u)$  of a generalized polynomial  $P_n$  which minimizes the weighted distance  $P(F, P_n)$  defined as

$$P(F, P_n) \equiv \left( \sum_{x \in M} W(x)(F(x) - P_n(x))^2 \right)^{1/2}, \quad \text{M discrete} \quad (I.1)$$

$$P(F, P_n) \equiv \left( \int_M W(x)(F(x) - P_n(x))^2 \right)^{1/2}. \quad \text{M compact} \quad (I.2)$$

The approximating polynomial is given by

$$P_n = \sum_{i=1}^n C_i \psi_i. \quad (I.3)$$

The scalar product of two functions  $G, H \in G_m$  is defined as

$$\langle G, H \rangle = \begin{cases} \sum_{x \in M} W(x).G(x).H(x), & \text{M discrete} \\ \int_M W(x).G(x).H(x). & \text{M compact} \end{cases} \quad (I.4)$$

If the product of two functions  $G, H \in G_m$  is zero, then the functions are orthogonal. If the base functions are orthogonal,

$$\langle \psi_i \psi_j \rangle = \begin{cases} k_i \neq 0 & i = j \\ 0 & i \neq j \end{cases} \quad (I.5)$$

If  $i = j$ , it means

$$k_i = \langle \psi_i \psi_i \rangle = \|\psi_i\|^2, \in E^+$$

or

$$\langle \psi_i \psi_i \rangle = \|\psi_i\|^2 \delta_{ij},$$

where  $\delta_{ij}$  is known as Kronecker delta and is defined as

$$\delta_{ij} = \begin{cases} 1 & i = j \\ 0 & i \neq j \end{cases} \quad (I.6)$$



Returning to the problem of least squares approximation we are seeking for the coefficients  $C_1, C_2 \dots C_u$  of the polynomial  $P_n$  that would make the distance  $||P_n - F||$  the minimum. This means minimizing the Euclidean distance

$$\sum_{X \in M} W(x)(F(x) - P_n(x))^2$$

with respect to  $C_1, C_2 \dots C_u$ . The condition is written as

$$\begin{aligned} & \text{Min} \\ & C_1, C_2, \dots, C_u \in E \quad \rho^2(P, F) \\ & = \text{Min} \\ & C_1, C_2, \dots, C_u \in E \quad \sum_{X \in M} W(x)(P_n(x) - F(x))^2 \\ & = \text{Min} \\ & C_1, C_2, \dots, C_u \in E \quad \sum_{X \in M} W(x) \sum_{i=1}^u (C_i \psi_i(x) - F(x))^2. \end{aligned} \quad (I.7)$$

When the partial derivatives of the above w.r.t individual  $C$ 's are equated to zero, the minimum distance is obtained.

Minimizing we have

$$\begin{aligned} & \frac{\partial}{\partial C_i} \sum_{X \in M} [W(x) \left( \sum_{j=1}^u C_j \psi_j(x) - F(x) \right)^2] \\ & = 2 \sum_{X \in M} W(x) \sum_{j=1}^u (C_j \psi_j(x) - F(x)) \frac{\partial \sum_{j=1}^u C_j \psi_j(x)}{\partial C_i} \\ & = 2 \sum_{X \in M} W(x) \sum_{j=1}^u C_j \psi_j(x) - F(x) \psi_i(x) \quad , \\ & = 2 \sum_X W(x) \sum_j C_j \psi_j(x) \psi_i(x) - 2 \sum_X W(x) F(x) \psi_i(x) \quad , \\ & = 0 \quad . \end{aligned} \quad (I.8)$$

From the definition of the scalar product, the above can be written as

$$\sum_{j=1}^u \langle \psi_i, \psi_j \rangle C_j = \langle F, \psi_i \rangle. \quad (\text{I.9})$$

Equation I.9 gives the system of normal equations which can be solved to obtain the coefficients  $C_1, C_2, \dots, C_u$ . Putting I.9 in matrix form we have

$$[\langle \psi_i, \psi_j \rangle] C = [\langle F, \psi_i \rangle]. \quad (\text{I.10})$$

Letting

$$N = [\langle \psi_i, \psi_j \rangle], \quad (\text{I.11})$$

and

$$U = [\langle F, \psi_i \rangle], \quad (\text{I.12})$$

the solution of normal equation is given by

$$C = N^{-1} U. \quad (\text{I.13})$$

$N$  is the Gram's matrix and Gram's determinant  $\det(N) \neq 0$  because we are dealing with linearly independent base functions  $\Psi$ . Equation I.13 therefore has a unique solution.

If we are dealing with orthogonal system of base functions  $\Psi^*$ , then

$$N^* = \text{Diag}[\langle \psi_i^*, \psi_i^* \rangle] = \text{Diag}(\|\psi_i^*\|^2).$$

The solution of the normal equation becomes trivial and is given by

$$C^* = \langle F, \psi_i^* \rangle / \|\psi_i^*\|^2, \quad i = 1, 2, \dots, u. \quad (\text{I.14})$$

Each Fourier coefficient  $C^*$  can be solved independently.

The system of base functions  $\Psi$  often encountered are not usually orthogonal. The system can however be orthogonalised using Schmidt's orthogonalization process. The

process works as follows:

i) choose

$$\psi_1^* = \psi_1 \quad X \in M \quad (\text{I.15})$$

ii) define

$$\psi_2^* = \psi_2 + \beta_{2,1} \psi_1^*, \quad X \in M, \beta_{2,1} \in E. \quad (\text{I.16})$$

Multiplying the above equation I.16 by  $W\psi_1^*$  and summing up all the equations for all the  $X$  yields

$$\langle \psi_2^*, \psi_1^* \rangle = \langle \psi_2, \psi_1^* \rangle + \beta_{2,1} \langle \psi_1^*, \psi_1^* \rangle. \quad (\text{I.17})$$

To make the system orthogonal,  $\langle \psi_2^*, \psi_1^* \rangle$  must be zero. The unknown coefficient  $\beta_{2,1}$  can be determined from

$$\beta_{2,1} = \langle \psi_2, \psi_1^* \rangle / \langle \psi_1^*, \psi_1^* \rangle. \quad (\text{I.18})$$

iii) Define next

$$\psi_3^* = \psi_3 + \beta_{3,2} \psi_2^* + \beta_{3,1} \psi_1^*, \quad X \in M, \beta_{3,2}, \beta_{3,1} \in E. \quad (\text{I.19})$$

Multiplying by  $W\psi_1^*$  and  $W\psi_2^*$  yield respectively

$$\langle \psi_3^*, \psi_1^* \rangle = \langle \psi_3, \psi_1^* \rangle + \beta_{3,2} \langle \psi_2^*, \psi_1^* \rangle + \beta_{3,1} \langle \psi_1^*, \psi_1^* \rangle,$$

$$\langle \psi_3^*, \psi_2^* \rangle = \langle \psi_3, \psi_2^* \rangle + \beta_{3,2} \langle \psi_2^*, \psi_2^* \rangle + \beta_{3,1} \langle \psi_1^*, \psi_2^* \rangle.$$

By reason of orthogonality,

$$\langle \psi_3^*, \psi_1^* \rangle = \langle \psi_2^*, \psi_1^* \rangle = \langle \psi_3^*, \psi_2^* \rangle = \langle \psi_1^*, \psi_2^* \rangle = 0.$$

We therefore have that

$$\langle \psi_3, \psi_1^* \rangle + \beta_{3,1} \langle \psi_1^*, \psi_1^* \rangle = 0,$$

$$\langle \psi_3, \psi_2^* \rangle + \beta_{3,2} \langle \psi_2^*, \psi_2^* \rangle = 0,$$

that is

$$\beta_{3,1} = \langle \psi_3, \psi_1^* \rangle / \langle \psi_1^*, \psi_1^* \rangle. \quad (I.20)$$

$$\beta_{3,2} = \langle \psi_3, \psi_2^* \rangle / \langle \psi_2^*, \psi_2^* \rangle. \quad (I.21)$$

The process can be generalized for any coefficient  $\beta_{ji}$  thus

$$\beta_{ji} = \langle \psi_j, \psi_i^* \rangle / \langle \psi_i^*, \psi_i^* \rangle. \quad (I.22)$$

Expressing the original system in terms of the orthogonal system we have

$$\psi_1 = \psi_1^*,$$

$$\psi_2 = -\beta_{2,1} \psi_1^* + \psi_2^*,$$

$$\psi_3 = -\beta_{3,1} \psi_1^* - \beta_{3,2} \psi_2^* + \psi_3^*,$$

⋮

$$\psi_u = -\beta_{u,1} \psi_1^* - \beta_{u,2} \psi_2^* \cdots - \beta_{u,u-1} \psi_{u-1}^* + \psi_u^*.$$

Putting it in matrix form we have

$$\begin{bmatrix} \psi_1 \\ \psi_2 \\ \psi_3 \\ \vdots \\ \psi_u \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 & \cdots & 0 \\ -\beta_{2,1} & 1 & 0 & \cdots & 0 \\ -\beta_{3,1} & \beta_{3,2} & 1 & \cdots & 0 \\ \vdots & \vdots & \vdots & \ddots & \vdots \\ -\beta_{u,1} & -\beta_{u,2} & \beta_{u,3} & \cdots & 1 \end{bmatrix} \begin{bmatrix} \psi_1^* \\ \psi_2^* \\ \psi_3^* \\ \vdots \\ \psi_u^* \end{bmatrix}. \quad (I.23)$$

$\beta_{ji}$  is defined by equation I.22.

Letting

$$B_{uxu} = \begin{bmatrix} 1 & 0 & 0 & \dots & 0 \\ -\beta_{2,1} & 1 & 0 & \dots & 0 \\ -\beta_{3,1} & -\beta_{3,2} & 1 & \dots & 0 \\ \vdots & \vdots & \vdots & \ddots & \vdots \\ -\beta_{u,1} & -\beta_{u,2} & -\beta_{u,3} & \dots & 1 \end{bmatrix},$$

equation I.23 is written as

$$\psi = B\psi^*. \quad (\text{I.24})$$

B is the transformation matrix that transforms non orthogonal system to orthogonal system. It is an  $u \times u$  triangular matrix and the determinant  $\det(B) \neq 0$ .

If we have that

$$\psi^T C = \psi^{*T} C^*,$$

using equation I.24, we can transform the Fourier coefficients into the coefficients of the original base functions, thus

$$\begin{aligned} (B\psi^*)^T C &= \psi^{*T} C^* \\ C &= (B^T)^{-1} C^*. \end{aligned} \quad (\text{I.25})$$

## II BRIEF DESCRIPTION OF THE COMPUTER PROGRAMS USED

The computer programs used in the computations are in three parts, namely

- (i) Least squares polynomial approximation for cotidal curves,
- (ii) Least squares polynomial approximation of observed time series at the reference station,
- (iii) Tidal reductions.

### II.1 Least Square Polynomial Approximation for the Cotidal Curves

Figure A-1 is the flow chart describing the program.

#### INPUTS

1st card, FORMAT(5X, 7I4)

ID - The dimension of the polynomial

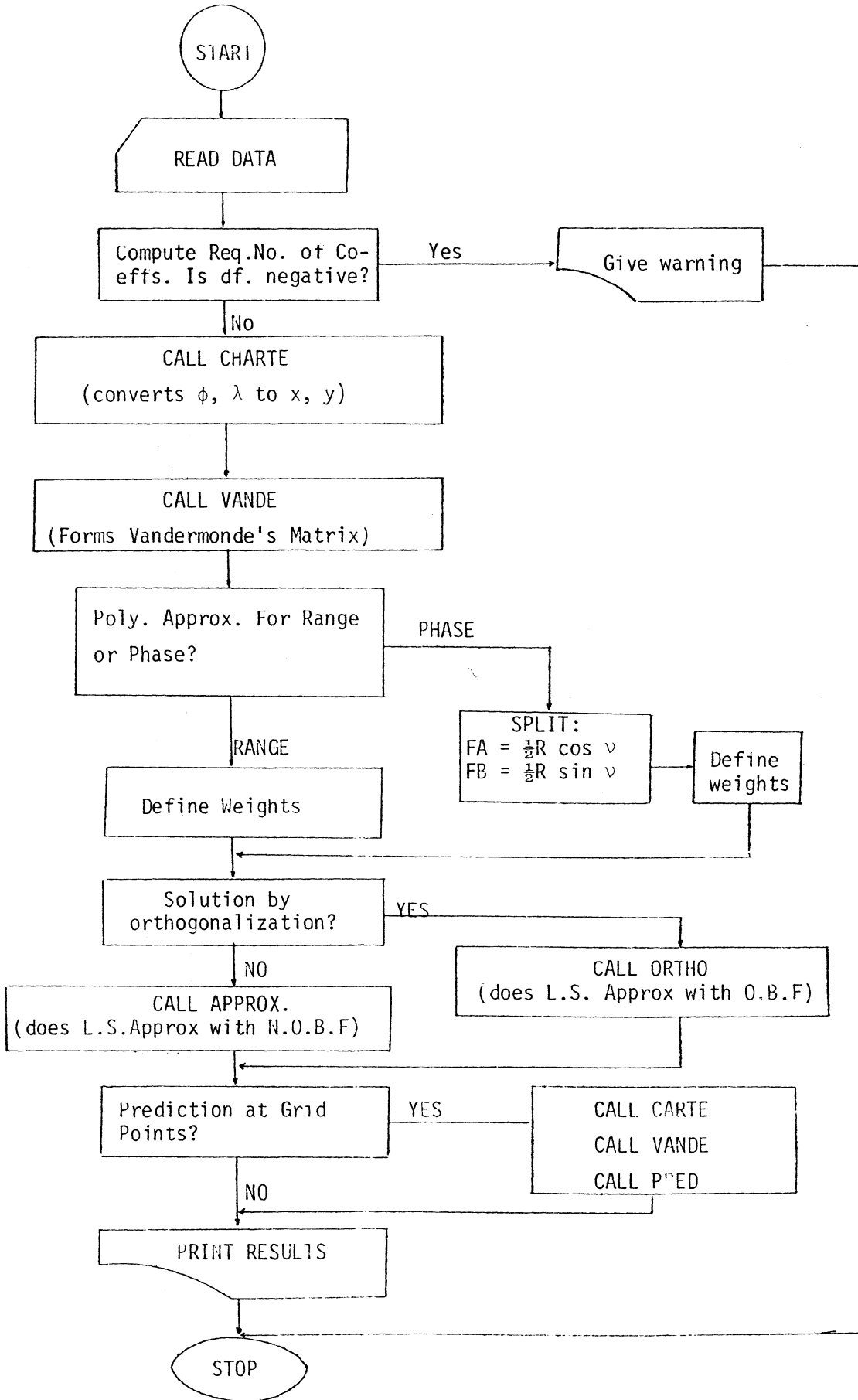
N - The degree of the polynomial

M - Number of data points for the approximation

NPP - Number of grid points for prediction. If there is no prediction NPP = 0

INDEX - Code for the type of function to be approximated. If index = 1, the polynomial approximation for range ratio (amplitude) is performed. If index = 2, the polynomial approximation for time lag (phase lag) is performed.

Figure II-1 Polynomial Approximation of Cotidal Curves - Flow Chart



ID - Code for orthogonal or non orthogonal solution, 1 - for orthogonal solution  
 2 - for non orthogonal solution.

ITEST - Code for testing Fourier coefficients  
 0 - for no test  
 1 - against its Standard Deviation  
 2 - against 2 times its Standard Deviation  
 3 - against 3 times its Standard Deviation.

2nd Card: FORMAT(5X, I5, 5X, F10.6, 5X, F10.6  
 5X, F10.6, 5X, F10.6)

This card contains the identification number, latitude  $\phi$  and longitude  $\lambda$  of the reference station, the range ratio (amplitude) and time lag (phase lag) at the reference station.

n cards: Format as in the second card. Each card contains, station identification number, latitude ( $\phi$ ), longitude ( $\lambda$ ) of the data points, the range ratio (amplitude diff.) and the time lag (phase lag diff.) at each data.

NPP cards: FORMAT(5X, I5, 5X, F10.6, 5X, F10.6)

If there are no prediction at the grid points, these cards will be omitted. Each card contains the grid point number, the geodetic coordinates of the grid



points  $(\phi, \lambda)$ .

**SUBROUTINES:**

SUBROUTINE CARTE; computes the local cartesian coordinates  $(x_i, y_i)$  given the geodetic coordinates of the points  $(\phi_i, \lambda_i)$ , the geodetic coordinates of the origin of the local system  $(\phi_0, \lambda_0)$  and the dimensions of the ellipsoid.

SUBROUTINE VANDE - computes the prediction matrix given the geodetic coordinates  $(\phi, \lambda)$  of the prediction points, the number of prediction points, the dimension of the polynomial and the number of coefficients.

SUBROUTINE APPROX. - does the Least Square approximation of the function given the number of coefficients, the number of data point, the Vandermonde's matrix, the weight matrix and the functional values.

SUBROUTINE ORTHO - orthogonalizes the Vandermonde's matrix using Gram Schmidt method, computes the Fourier Coefficients of the orthogonalized matrix, derives the coefficients of the Vandermonde's matrix, computes the variances of the Fourier Coefficients and the variance-covariance matrix of the original coefficients.

SUBROUTINE PRED. - predicts the function values at the grid points and computes the variance-covariance matrix of the prediction.

```

*****
3JUB CKENW/G
C *****
C * PROGRAM TO CONSTRUCT ANALYTICAL COTIDAL CHARTS USING ANALYSED *
C * TIDAL CONSTANT OR RANGE RATIOS AND TIME DIFF. REF. TO A STANDAR *
C * D STATION OR DIGITIZED VALUES FROM EXISTING COTIDAL CHART *
C *
C * THIS USES THE LAT. AND LONG. OF DISCRETE POINTS, THE AMPL. AND *
C * PHASE LAGS OR RANGE RATIOS AND TIME DIFFS. AT THE POINTS AS *
C * THE INPUTS TO COMPUTE THE COEFFICIENTS OF THE BEST PREDICTING *
C * POLYNOMIALS AT ANY OTHER POINT. *
C *
C * GUIDE TO SOME NOTATIONS USED IN THE PROGRAM *
C * INPUT: *
C * FHI - LATITUDE OF STN. I *
C * ALON - LONGITUDE OF STN. I *
C * FH,FG - AMPLITUDE AND PHASE LAG OR RANGE RATIO AND TIME *
C *          DIFF. IN MINUTES OF TIME. *
C * ID - DIMENSION OF THE APPROX. *
C * N - DEGREE OF THE POLYNOMIAL *
C * M - NUMBER OF OBSERVATIONS *
C * IC - CODE FOR ORTHOGONAL OR NON ORTHOGONAL SOLUTION. *
C *       1 - FOR ORTHOGONAL SOL. *
C *       2 - FOR NON ORTHOGONAL SOL. *
C * ITEST : CODE FOR TESTING FOURIER COEFFS. *
C *       0 - NO TEST *
C *       1 - AGAINST ITS STD. DEV. *
C *       2 - AGAINST 2 TIMES ITS STD. DEV. *
C *       3 - AGAINST 3 TIMES ITS STD. DEV. *
C * NPP: - NO. OF GRID POINTS FOR PREDICTION. IF THERE IS *
C *       NO PREDICTION AT GRID POINTS NPP=0 *
C *
C * OUTPUT: *
C * L - NO. OF COEFFICIENTS REQUIRED. (WHEN THE NO. OF COEFF *
C *   EXCEED THE NO. OF OBS THE PROGRAM IS ABORTED) *
C * C,CA,CE - VECTORS OF COEFFICIENTS. *
C *
C * SEE SUBROUTINE ORTHO FOR MORE EXPLANATIONS OF OUTPUT NOTATIONS *
C *
C *****
C * MAIN PROGRAME
1      IMPLICIT REAL*8(A-H,O-Z)
2      DIMENSION PHI(50),ALON(50),FH(50),FG(50),A(50,50),P(50,50)
*      ,COVAR(50,50),VAR(50,50),C(50),EN(50,50),X(50),Y(50),V(50),
*      U(50),NUM(50),NGRID(50),XP(50),YP(50),PMT(50,50),PMCO(50,50)
*      ,AC(50),VARP(50)
3      DIMENSION ALAT(50),ALONG(50),PM(50,50),PFH(50),PFG(50)
4      DIMENSION ALPHA(50,50),w(50),FC(50),SUMFC(50),SGN(50),STDP(50),
*      D(50),SIGMAF(50),SIGMAR(50)
5      DIMENSION FA(50),FB(50),FA2(50),FB2(50),WA(50),WB(50),CA(50),
*      CB(50),CLA(50,50),COB(50,50),PFA(50),PFB(50),VARPA(50,50),
*      VARPB(50,50),B(2,2),AR(50,50),FR(50)
6      IRCA=ICA=ICE=ICC=50
7      #HC=206265.000
8      PI=3.141592653589793

```

```

9      IW=6
10     SIGMA=1.000

C
C     READ IN PROGRAM SPECIFICATION
C
11     HEAD(5,146)ID,N,M,NPP,INDEX,IO,ITEST
12     146   FCRMAT(5),7I4)

C
C     COMPLETE NO. OF COEFFICIENTS AND DEGREE OF POLY AND DEGREE OF
C     OF FREEDOM. IF DF IS ZERO OR NEGATIVE GIVE WARNING AND STOP
C
13     L=(N+1)**IO
14     IDF=M-L
15     IF(100*LE,0)THEN DO
16     PRINT,'PROGRAM SPECIFICATION INADEQUATE'
17     STOP
18     ELSE DO
19     END IF
20     CONTINUE

C
C     READ IN DATA ON THE STANDARD OR REF. STATION
C
21     READ(5,200)NUMC,PHIO,ALONC,FHO,FGO
22     200   FCRMAT(5X,15,5X,F10.6,5X,F10.6,5X,F10.6,5X,F10.6)
23     100   FCRMAT(5X,15,5X,F10.6,5X,F10.6)

C
C     READ IN DATA FROM OTHER STATIONS
C
24     DO 1 I=1,M
25     READ(5,200)NUM(I),PHI(I),ALON(I),FH(I),FG(I)
26     1     CONTINUE

C
C     PRINT ALL THE INPUT DATA
C
27     PRINT101
28     101   FCRMAT(//,4X,'NO',8X,'LATITUDE',10X,'LONGITUDE',10X,'RANGE RA
* TIO',10X,'TIME LAG')
29     102   FCRMAT(5X,14,7X,F10.6,9X,F10.6,9X,F10.6,9X,F10.6)
30     PRINT102,NUMC,PHIO,ALONC,FHO,FGO
31     DO 3 I=1,M
32     PRINT102,NUM(I),PHI(I),ALON(I),FH(I),FG(I)
33     3     CONTINUE

C
34     PRINT131
35     131   FCRMAT(//,5X,'DEG. OF POLY.',5X,'NO. OF OBS.',5X,'NO. OF COEFF.',
*      5X,'DEG. OF FREEDOM')
36     PRINT132,N,M,L,IDF
37     132   FCRMAT(//,8X,I2,12X,I3,15X,I3,20X,I3)
38     PRINT130
39     130   FCRMAT(//,5X,'CARTESIAN COORD. OF THE GIVEN STATIONS')
40     PRINT127
41     127   FCRMAT(//,5X,'STN. NO.',5X,'X-COORD.',15X,'Y-COORD.')
42     CALL CARTE(N,PHI,ALON,PHIO,ALONO,X,Y)
43     DO 4 I=1,M
44     PRINT111,NUM(I),X(I),Y(I)
45     111   FCRMAT(/,6X,I4,4X,F15.6,7X,F15.6)
46     4     CONTINUE

C
C     COMPLETE FOR A MATRIX

```

```

47      C      DO 5 I=1,M
48      DO 5 J=1,L

49      A(I,J)=C.DO
50      CONTINUE
51      5      IDF=N+1
52      DO 7 IRDF=1,M
53      ICCP=C
54      DO 13 I=1,ICF
55      IA=I-1
56      DO 14 J=1,IDP
57      JA=J-1
58      ICCP=ICCP+1
59      A(IRDF,ICDP)=X(IRDF)**IA*Y(IRDF)**JA
60      14      CCNTINCE
61      13      CONTINUE
62      7      CONTINUE
63      PRINT128
64      128     FORMAT(//,5X,'VENDEMCND MATRIX')
65      PRINT,'
66      CALL MCLTD(A,ICA,M,L)

C
C      DETERMINE THE VALUES AT THE DISCRETE POINTS
C
67      IF(NFP.EG.C) GO TO 97
68      DO 44 I=1,NFP
69      READ(5,100)NGRID(I),ALAT(I),ALCNG(I)
70      44      CONTINUE
71      CALL CARTE(NPP,ALAT,ALLNG,PHIO,ALCNO,XP,YP)
72      CALL VANCE(NPP,L,IDP,ALAT,ALCNG,PHIO,ALCNO,XP,YP,PM)
73      97      CONTINUE
74      DO 15 ISIG=1,INDEX
75      IF(ISIG.EG.1)THEN DO
76      PRINT,'++ POLYNOMIAL APPROXIMATION FOR RANGE RATIO'
77      PRINT103
78      103     FORMAT(4X,'*****')

C
C      IF AMPLITUDE OF RANGE RATIO IS TO APPROXIMATED PROCEED,
C      OTHERWISE GO TO STATEMENT NC 46
C
79      IF(IC.EG.1)GO TO 46
80      C      FORMATION OF WEIGHT MATRIX
81      DO 8 I=1,M
82      DO 8 J=1,M
83      F(I,J)=C.DO
84      8      CONTINUE
85      DO 9 I=1,M
86      J=I
87      P(I,J)=100.CDO
87      9      CONTINUE

C
C      PERFORM THE LEAST SQUARES APPROX BY CALLING THE SUBROUTINE APPROX
C
88      CALL APPROX(L,M,A,P,PH,EN,C,V,AC,U,CCVAR,APVF)
89      PRINT105
90      105     FORMAT(10X,'VECTOR OF CCEFFICIENTS' )
91      CALL MCLTD(C,ICA,L,1)
92      PRINT106
93      106     FORMAT(10X,'RESIDUAL',5X,'VECTOR' )

```

```

94      CALL MCLTD(V,IDA,M,1)
95      PRINT107,APVF
96      107  FORMAT(10X,'A POSTERIORI VARIANCE FACTOR =',F10.6)
97      PRINTICE

98      108  FORMAT(10X,'VARIANCE COVARIANCE MATRIX OF THE COEFFICIENTS')
99      CALL MCLTD(CCVAR,IDA,L,L)
100     PRINT110
101     110  FCFORMAT(//,5X,'POLYNOMIAL APPROX OF THE GIVEN FUNCTIONS')
102     CALL MCLTD(AC,IDA,M,1)
103     IF(IC.EQ.2)GO TO 51
104     4E   CCNTINUE
105     DO 57 I=1,M
106     W(I)=100.0DC
107     57   CCNTINUE

C
C   PERFORM LEAST SQUARES APPROX USING ORTHOGONAL BASE FUNCTIONS
C   BY CALLING THE SUBROUTINE ORTHO
C
108     CALL ORTHO(M,L,SIGMA,A,IDA,SIGMFC,VFC,NPC,ITEST,V,COVAR,FH,
109     *     W,C,ALPHA,FC,SUMFC,SN,STDP,IW)
110     PRINT113
111     DO 47 I=1,L
112     PRINT1134,FC(I),SUMFC(I)
113     47   CCNTINUE
114     PRINT1135
115     DO 4E I=1,L
116     PRINT1136,C(I)
117     4E   CCNTINUE
118     PRINT,'NO. OF COEFF. OF ORIGN. POLY. AFTER TEST =',NPC
119     PRINT1137
120     CALL MCLTD(CCVAR,IDA,L,L)
121     PRINT1138
122     DO 49 I=1,M
123     PRINT1139,V(I)
124     49   CCNTINUE
125     PRINT,' A POSTERIORI VARIANCE FACTOR= ',VFC
126     51   CCNTINUE

C
C   IF PREDICTION IS REQUIRED CALL THE SUBROUTINE PRED
C
126     IF(NFF.EQ.0)GO TO 98
127     CALL PRED(NPP,L,PM,C,COVAR,PMT,PH,PMCO,VAR)
128     ELSE DO

C
C   IF APPROX IS FOR TIME LAG (PHASE LAG)PROCEED
C
129     PRINT,'44 POLYNOMIAL APPROXIMATION FOR TIME LAG'
130     PRINT1103
131     CK=0.5*F1/180.
132     FO=25.10/3.2808
133     ER=0.125*PI/180.
134     ERF=C.10
135     DO 60 I=1,M
136     FH=FC(I)*CK
137     R=FO*FH(I)/2.
138     FA(I)=R*CCOS(PH)
139     FB(I)=R*CSIN(PH)
140     WA(I)=1./ERF**2

```

```

141      WB(I)=1./ERR**2
      C
      C   FOR ORTHOGONAL BASE FUNCTIONS STATEMENT 52 IS EXECUTED
      C
142      GO      CONTINUE

      C
143      IF(IC.EG.1)GO TO 52
144          DO 17 I=1,M
145          DO 17 J=1,M
146          F(I,J)=C.CO
147      17      CONTINUE
148          DO 18 I=1,M
149          J=1
150          P(I,J)=WA(I)
151      18      CONTINUE
152          PRINT140
153          CALL APPROX(L,M,A,P,FA,EN,CA,V,AC,U,CCA,APVF)
154          PRINT105
155          CALL MCLTD(CA,IDA,L,1)
156          PRINT106
157          CALL MCLTD(V,ICA,M,1)
158          PRINT107,APVF
159          DO 16 I=1,M
160          DO 16 J=1,M
161          F(I,J)=0.000
162      16      CONTINUE
163          DO 19 I=1,M
164          J=1
165          P(I,J)=WB(I)
166      19      CONTINUE
167          PRINT135
168          CALL APPROX(L,M,A,P,FB,EN,CB,V,AC,U,CUB,APVF)
169          PRINT105
170          CALL MCLTD(CB,ICA,L,1)
171          PRINT106
172          CALL MCLTD(V,ICA,M,1)
173          PRINT107,APVF
174          IF(IC.EG.2)GO TO 56
175      52      CONTINUE
      C   FIND POLYNOMIAL APPROX. FOR FUNCTIONS FA,FB
      C
176          PRINT140
177      140      FORMAT(//,5X,'POLYNOMIAL APPROXIMATION FOR FUNCTION FA')
178          CALL LRTHO(M,L,SIGMA,A,IDA,SIGMFC,VFC,NPC,ITEST,V,COA,FA,WA,CA,
      *          ALPHA,FC,SUMFC,SGN,STDP,IW)
179          PRINT133
180          DO 53 I=1,L
181          PRINT134,FC(I),SUMFC(I)
182      53      CONTINUE
183          PRINT135
184          DO 54 I=1,L
185          PRINT136,CA(I)
186      54      CONTINUE
187          PRINT,'NO. OF COEFF. OF ORIGN. POLY. AFTER TEST =',NPC
188          PRINT137
189          CALL MCLTD(CCA,IDA,L,L)
190          PRINT138
191          DO 55 I=1,M
192          PRINT139,V(I)

```

```

193      55      CONTINUE
194      PRINT,'A POSTECRI VARIANCE FACTOR=',VFC
C
195      PRINT135
196      135      FORMAT(//,5X,'POLYNOMIAL APPROXIMATION FOR FUNCTION FB')
C

197      *      CALL LRTHO(N,L,SIGMA,A,IDA,SIGMFC,VFC,NPC,ITEST,V,COB,FB,WB,CB,
198      *      ALPHA,FC,SUMFC,SGN,STDP,IW)
199      PRINT133
200      DO 61 I=1,L
201      PRINT134,FC(I),SUMFC(I)
202      61      CONTINUE
203      PRINT135
204      DO 62 I=1,L
205      PRINT136,CE(I)
206      62      CONTINUE
207      PRINT,'NC OF POLY.AFTER TEST=',NFC
208      PRINT137
209      CALL MOUTD(CCE,ICA,L,L)
210      PRINT138
211      DO 63 I=1,M
212      PRINT136,V(I)
213      63      CONTINUE
214      PRINT,'A POSTECRI VARIANCE FACTOR=',VFC
C
215      56      CONTINUE
C
C      IF THERE IS NO PREDICTION STATEMENT NO 98 IS EXECUTED
C
216      IF(NFP.EQ.0) GO TO 98
C      PREDICTION OF FUNCTIONS FA,FB AT GRID POINTS
217      CALL FRED(NFP,L,FM,CA,CCA,FMT,PFA,PMCC,VARPA)
C
C      CALL FRED(NFP,L,FM,CE,CCE,FMT,PFE,PMCG,VARPB)
C
C      COMPUTE THE PREDICTED TIME LAG AND ASSOCIATED VARIANCES
218      DO 64 I=1,NFP
219      PFC(I)=DATAN(PFB(I)/PFA(I))/CK
220      E(1,1)=(1./((1.+(PFB(I)/PFA(I))**2)))*(-PFB(I)/PFA(I)**2)
221      E(1,2)=(1./((1.+(PFB(I)/PFA(I))**2)))*(1./PFA(I))
C      COMPUTE VARIANCES
222      VARP(I)=E(1,1)**2*VARPA(I,1)+E(1,2)**2*VARPB(I,1)
223      SIGMAF(I)=DSGRT(VARP(I))/CK
224      64      CONTINUE
225      END IF
226      98      CONTINUE
227      15      CONTINUE
228      PRINT,'*** PREDICTION      MATRIX      ***'
229      CALL MOUTD(FM,ICA,NFP,L)
230      PRINT126
231      PRINT127
232      126      FORMAT(//,5X,'CARTESIAN COORDS. OF GRID FUINTS')
233      DO 50 I=1,NFP
234      PRINT111,NGRID(I),XP(I),YP(I)
235      50      CONTINUE
236      PRINT125
237      125      FORMAT('1',//,5X,'PREDICTED RANGE RATIOS AND TIME LAGS AT THE G
238      *RID PCINTS')
      PRINT141

```

```

239          DO 45 I=1,NFP
240          ALAT(I)=ALAT(I)*180./PI
241          ALONG(I)=ALONG(I)*180./PI
242          SIGMAR(I)=DSQRT(VAR(I,I))
243          PHIR=INITI42,NGRID(I),ALAT(I),ALONG(I),PFH(I),SIGMAR(I),PFG(I),
*          SIGMAR(I)
244          45      *          CONTINUE

245          DO 66 I=1,L
246          J=I
247          WRITE(7,145)((I),CCVAR(I,J)
248          66      *          CONTINUE
249          DO 67 I=1,L
250          J=I
251          WRITE(7,145)(A(I),COA(I,J)
252          67      *          CONTINUE
253          DO 68 I=1,L
254          J=I
255          WRITE(7,145)(B(I),CCB(I,J)
256          68      *          CONTINUE
257          133      *          FCFMAT(//,5X,'FOURIER COEFFICIENT',10X,'VARIANCES')
258          134      *          FCFMAT(/,8X,E11.4,10X,E11.4)
259          135      *          FCFMAT(//,5X,'VECTOR OF ORIGINAL COEFFICIENTS')
260          136      *          FCFMAT(/,5X,E11.4)
261          137      *          FCFMAT(//,5X,'VARIANCE-COVARIANCE MATRIX OF THE COEFF.')
262          138      *          FCFMAT(//,5X,'VECTOR OF RESIDUALS')
263          141      *          FCFMAT(/,4X,'NO',8X,'LATITUDE',8X,'LONGITUDE',10X,'RANGE RATIO
*          ',4X,'SIGMA',7X,'TIME LAC',5X,'SIGMA')
264          142      *          FCFMAT(/,3X,14,5X,F10.6,6X,F10.6,6X,F10.6,6X,E11.4,6X,F10.6,
*          6X,E11.4)
265          145      *          FCFMAT(5X,2E11.4)
266          STOP
267          END

C
268          SUBROUTINE CARTE(M,ALAT,ALON,PHIC,ALCNO,X,Y)
C
C          THIS SUBROUTINE COMPUTES THE CARTESIAN COORDS. X,Y, FROM GEOGRAPHICAL
C          COORDS.,LATITUDE AND LONGITUDE.,
C
C
269          IMPLICIT REAL*8(A-H,O-Z)
270          DIMENSION ALAT(50),ALON(50),X(50),Y(50)
271          RA=6378206.400
272          RB=6356583.800
273          PI=3.141592653589793
274          EC=(RA**2-RB**2)/RA**2
275          PHIR=PHIC*PI/180.
276          ALCNR=ALCNO*PI/180.
277          XM=((1.-EC)*RA)/(DSQRT(1.-EC*(DSIN(PHIR)**2))**3)
278          XN=RA/DSQRT(1.-EC*(DSIN(PHIR)**2))
279          R=DSQRT(XM**2+XN**2)
280          DO 4 I=1,M
281          ALAT(I)=ALAT(I)*PI/180.
282          ALON(I)=ALON(I)*PI/180.
283          X(I)=R*(ALAT(I)-PHIR)
284          Y(I)=R*CCOS(PHIR)*(ALON(I)-ALCNR)
285          4      *          CONTINUE
286          RETURN
287          END

```



```

C
288      SUBROUTINE VANDE(NPP,L,IDP,ALAT,ALCNG,PHIO,ALCND,XP,YP,PM)
C      THIS SUBROUTINE COMPUTES THE PREDICTION MATRIX PM,
C
C
289      IMPLICIT REAL*8(A-H,O-Z)
290      DIMENSION PM(50,50),ALAT(50),ALCNG(50),XP(50),YP(50)

291      DO 40 I=1,NPP
292      DO 40 J=1,L
293      FM(I,J)=0.CDO
294      40 CONTINUE
295      DO 41 I=1,NPP
296      ICDF=0
297      DO 42 K=1,IDP
298      KA=K-1
299      DO 43 J=1,IDP
300      JA=J-1
301      ICDF=ICDF+1
302      FM(I,ICDF)=XP(I)**KA*YF(I)**JA
303      43 CONTINUE
304      42 CONTINUE
305      41 CONTINUE
306      RETURN
307      END

```

```

C
308      SUBROUTINE PRED(NPP,L,PM,C,COVAR,PMT,PF,PMCO,VAR)
C      THIS SUBROUTINE PREDICTS THE FUNCTION VALUES AT THE GRID POINTS
C      AND COMPUTES THE PREDICTION VARIANCE COVARIANCE MATRIX.
C
C

```

```

309      IMPLICIT REAL*8(A-H,O-Z)
310      DIMENSION FM(50,50),C(50),PF(50),PMCO(50,50),COVAR(50,50)
*      ,VAR(50,50),PMT(50,50)
311      IRCA=ICA=ICB=IDC=50
312      CALL MMULD(PF,IDC,PM,IDA,C,IDB,NPP,L,1)
313      CALL THNSD(FMT,IDE,PM,IDA,NPP,L)
314      CALL MMULD(PMCO,IDC,PM,IDA,COVAR,IDB,NPP,L,L)
315      CALL MMULD(VAR,IDC,PMCO,IDA,PMT,IDB,NPP,L,NPP)
316      RETURN
317      END

```

```

318      SUBROUTINE APPROX(L,M,A,F,F,EN,C,V,AC,U,COVAR,APVF)
C      *****
C      *      THIS SUBROUTINE DOES THE LEAST SQUARES APPROXIMATION *
C      *      OF THE GIVEN FUNCTIONS AND RETURNS THE VECTOR OF COEF *
C      *      FICIENTS TOGETHER WITH THE VAR. COVAE MATRIX *
C      *****
C

```

```

319      IMPLICIT REAL*8(A-H,O-Z)
320      DIMENSION A(50,50),P(50,50),EN(50,50),AC(50),C(50),F(50),
*      COVAR(50,50),ATP(50,50),AT(50,50),VTP(1,50),VF(1),U(50),
*      IW1(50),IW2(50),VT(1,50),V(50,1)
321      IRCA=ICA=ICB=IDC=50
322      CALL THNSD(AT,IDE,A,IDA,M,L)
323      CALL MMULD(ATP,IDC,AT,IDA,F,IDB,L,M,M)
324      CALL MMULD(EN,IDC,ATP,IDA,A,IDB,L,M,L)
325      CALL MMULD(VT,IDC,ATP,IDA,F,IDB,L,M,L)

```

```

326          DO 5 I=1,L
327          DO 5 J=1,L
328          EN(I,J)=EN(I,J)*1.0D-20
329          CONTINUE
330          CALL MINVD(EN,IRDA,L,DETA,IW1,IW2)
331          DO 6 I=1,L
332          DO 6 J=1,L
333          EN(I,J)=EN(I,J)*1.0D-20
334          CONTINUE

335          CALL MMULD(C,IDC,EN,IDA,U,IDB,L,L,1)
336          C      COMPUTE RESEALS
337          CALL MMULD(AC,IDC,A,IDA,C,IDC,M,L,1)
338          CALL MSUED(V,IDC,AC,IDA,F,IDC,M,1)
339          C      COMPUTE A FOSTERICKI VARIANCE FACTOR
340          CALL TRNSD(VT,1,V,IDA,M,1)
341          CALL MMLLD(VIF,1,VT,1,P,IDB,1,M,M)
342          CALL MMLLD(VF,1,VTP,1,V,IDB,1,M,1)
343          IDF=M-L
344          APVF=VF(1)/IDF
345          C      COMPUTE VARIANCE COVARIANCE MATRIX OF COEFFICIENT
346          DO 10 I=1,L
347          DO 10 J=1,L
348          CCVAF(I,J)=APVF*EN(I,J)
349          CONTINUE
350          RETURN
351          END

352          SUBROUTINE CHCLD(A,IRDA,NA,DETA,*)
353          C      THE USE OF THIS SUBROUTINE IS CRITICAL
354          C      MATRIX INVERSION USING CHOLESKI DECOMPOSITION
355          C      INPUT ARGUMENTS
356          C      A = ARRAY CONTAINING POSITIVE DEFINITE SYMMETRIC INPUT MATRIX
357          C      IRDA = ROW DIMENSION OF ARRAY CONTAINING INPUT MATRIX
358          C      NA = SIZE OF INPUT MATRIX
359          C      OUTPUT ARGUMENTS
360          C      DETA = DETERMINANT OF INPUT MATRIX
361          C      A = CONTAINS INVERSE OF INPUT MATRIX (INPUT DESTROYED)
362          C      * = ERROR RETURN (TAKEN IF NA .LT. 1 OR IF DETA .LT. SING)
363          C
364          DOUBLE PRECISION A,DETA,SUM,SCRT,DSCRT,ABS,DABS,SING
365          DIMENSION A(IRDA,NA)
366          SCRT(SUM)=DSQRT(SUM)
367          ABS(DETA) = DABS(DETA)
368          DATA SING/1D-50/
369          C      CHOLESKI DECOMPOSITION OF INPUT MATRIX INTO TRIANGULAR MATRIX
370          IF(NA .LT. 1) GO TO 18
371          DETA = A(1,1)
372          A(1,1) = SCRT(A(1,1))
373          IF(NA .EQ. 1) GO TO 6
374          DO 1 1 = 2,NA
375          A(1,1) = A(1,1) / A(1,1)
376          DO 5 J = 2,NA
377          SUM = 0.
378          J1 = J - 1
379          DO 2 K = 1,J1
380          SLM = SLM + A(J,K) ** 2

```

```

00007260
00007270
00007280
00007290
00007300
00007310
00007320
00007330
00007340
00007360
00007350
00007370
00007380
00007390
00007400
00007410
00007420
00007430
00007440
00007450
00007460
00007470
00007480
00007490
00007500
00007510
00007520
00007530
00007540
00007550

```

```

366      A(J,J) = SQRT(A(J,J) - SUM)                                00007570
367      DELTA = DELTA * (A(J,J) - SUM)                            00007560
368      IF(J .EQ. NA) GO TO 5                                     00007580
369      J2 = J + 1                                               00007590
370      DO 4 I = J2,NA                                           00007600
371          SUM = 0.                                              00007610
372          DO 3 K = 1,J1                                         00007620
373              SUM = SUM + A(I,K) * A(J,K)                       00007630
374          A(I,J) = (A(I,J) - SUM) / A(J,J)                     00007640
375      CONTINUE                                                  00007650

376      6 IF(ABS(DELTA) .LT. SING) GO TO 16                       00007660
C INVERSION OF LOWER TRIANGULAR MATRIX                            00007670
377      DO 7 I = 1,NA                                           00007680
378          A(I,I) = 1. / A(I,I)                                   00007690
379      7 CONTINUE
380      IF(NA .EQ. 1) GO TO 10                                     00007700
381      N1 = NA - 1                                               00007710
382      DO 5 J = 1,N1                                             00007720
383          J2 = J + 1                                             00007730
384          DO 5 I = J2,NA                                         00007740
385              SUM = 0.                                           00007750
386              I1 = I - 1                                          00007760
387              DO 8 K = J,I1                                       00007770
388                  SUM = SUM + A(I,K) * A(K,J)                   00007780
389                  A(I,J) = - A(I,I) * SUM                       00007790
C CONSTRUCTION OF INVERSE OF INPUT MATRIX                        00007800
390      10 DO 15 J = 1,NA                                         00007810
391          IF(J .EQ. 1) GO TO 12                                  00007820
392          J1 = J - 1                                             00007830
393          DO 11 I = 1,J1                                         00007840
394              A(I,J) = A(I,I)                                     00007850
395          12 DO 14 I = J,NA                                       00007860
396              SUM = 0.                                           00007870
397              DO 13 K = 1,NA                                       00007880
398                  SUM = SUM + A(K,I) * A(K,J)                   00007890
399              A(I,J) = SUM                                       00007900
400          15 CONTINUE                                           00007910
401          RETURN                                               00007920
402      16 WRITE(6,17) DELTA                                       00007930
403      17 FORMAT(10X, 'SINGULAR MATRIX IN CHOLD. DET =',E20.5)  00007940
404          RETURN 1                                             00007950
405      18 WRITE(6,19)                                             00007960
406      19 FORMAT(10X, 'MATRIX OF DIMENSION ZERO IN CHOLD')     00007970
407          RETURN 1                                             00007980
408      END                                                       00007990

409      SUBROUTINE CRTFC(N,N,SIGMA,PHI,MRD,SIGMAF,VFC,NPC,INDEX,V,SUMD,F,W
      & ,ALPHA,C,SUMC,SC2,STDP,1W)                                00007370
C THIS SUBROUTINE ORTHOGONALIZES THE MATRIX PHI USING THE GRAM-SCHMIDT 00007380
C METHOD, COMPUTES THE FOURIER COEFFICIENTS OF THE ORTHOGONALIZED MATRIX 00007390
C DERIVES THE COEFFICIENTS OF PHI, COMPUTES THE VARIANCES OF THE FOURIER 00007400
C COEFFICIENTS AND THE VARIANCE-COVARIANCE MATRIX OF THE COEFFICIENTS 00007410
C INPUTS :                                                       00007420
C 1. PHI(OPTIONAL - COULD BE FUNCTION SUBPROGRAM INSTEAD) - AN N BY M 00007430
C    CONTAINING THE BASE FUNCTIONS EVALUATED FOR EACH OBSERVATION 00007440
C 2. N - THE NUMBER OF OBSERVATIONS 00007450
C 3. M - THE NUMBER OF BASE FUNCTIONS (EQUAL OR GREATER THAN 2) 00007460
C 4. W - A VECTOR OF LENGTH N CONTAINING THE COMPUTED WEIGHT FUNCTIONS 00007470
C    (OPTIONAL VALUES) 00007480

```

```

C 6. SIGMA - THE A PRIORI VARIANCE FACTOR 00007500
C 7. MRC - THE MAXIMUM ROW DIMENSION OF PHI 00007510
C 8. MRC - THE MAXIMUM COLUMN DIMENSION OF PHI 00007520
C 9. INDX - PERMITS OPTIONAL TEST FOR STATISTICAL SIGNIFICANCE 00007530
C OF FOURIER COEFFICIENTS.... 00007540
C IF 0, STATISTICAL TEST FOR FOURIER COEFFICIENTS ABANDONED 00007550
C IF 1, TESTS AGAINST ONE TIME ITS STANDARD DEVIATION 00007560
C IF 2, TESTS AGAINST TWICE ITS ST. DEVIATION 00007570
C IF 3, TESTS AGAINST THREE TIMES ITS ST. DEVIATION 00007580
410 C IMPLICIT REAL*8(A-H,O-Z) 00007750
C 10. IW - WRITE CODE OF THE COMPUTER 00007590

C OUTPUTS : 00007600
C 1. ALPHA - AN MRC BY M MATRIX CONTAINING THE ALPHA'S USED IN COMPUTING 00007610
C THE ORTHOGONALIZED MATRIX AND IN COMPUTING THE COEFFICIENTS OF PHI 00007620
C 2. C - THE M FOURIER COEFFICIENTS OF THE ORTHOGONALIZED MATRIX 00007630
C 3. D - THE M COEFFICIENTS OF THE INPUT MATRIX PHI 00007640
C 4. SUMC - THE VARIANCES OF THE FOURIER COEFFICIENTS 00007650
C 5. SUMD - THE VARIANCE-COVARIANCE MATRIX OF THE COEFFICIENTS 00007660
C 6. SC2 - THE SQUARES OF THE NORMS OF THE ORTHOGONALIZED MATRIX 00007670
C 7. SIGMAP - THE FOURIER POLYNOMIAL A POSTERIORI VARIANCE FACTOR 00007680
C 8. V - THE N RESIDUALS 00007690
C 9. VFC - THE ORIGINAL POLYNOMIAL A POSTERIORI VARIANCE FACTOR 00007700
C 10. NPC - NUMBER OF THE COEFFICIENTS OF THE ORIGINAL POLYNOMIAL 00007710
C AFTER THE STATISTICAL TEST IS PERFORMED 00007720
C 11. STOP - VECTOR AGAINST WHICH THE ABSOLUTE VALUES OF FOURIER 00007730
C COEFFICIENTS ARE TESTED 00007740
411 DIMENSION ALPHA(50,50),W(50),F(50),C(50),D(50)
412 DIMENSION SUMD(50,50),SUMC(50),SC2(M),V(50),STDP(50),
* PHI(50,50)
C TEST FOR NEGATIVE DEGREES OF FREEDOM 00007800
413 IF (N.LT.M) GO TO 100 00007810
414 K=1 00007820
415 ALPHA(N,M)=1.00 00007830
C DETERMINE THE ALPHA'S FOR COMPUTATION OF ORTHOGONALIZED MATRIX 00007840
416 10 DO 3 J=K,M 00007850
417 IF (J.NE.K) GO TO 6 00007860
418 ALPHA(K,K)=1.00 00007870
419 GO TO 3 00007880
420 6 SC1=0.00 00007890
421 SC2(K)=0.00 00007900
422 SC3=C.00 00007910
423 DO 2 I=1,N 00007920
424 P=PHI(I,K) 00007940
425 IF (K.EQ.1) GO TO 4 00007950
426 K1=K-1 00007960
427 DO 5 J1=1,K1 00007970
428 F=P+ALPHA(J1,K)*PHI(I,J1) 00007980
429 4 SC1=SC1+W(I)*PHI(I,J1)*P 00007990
430 SC3=SC3+F(I)*W(I)*P 00008000
431 2 SC2(K)=SC2(K)+W(I)*P**2 00008010
432 ALPHA(J,K)=-SC1/SC2(K) 00008020
433 ALPHA(K,J)=ALPHA(J,K) 00008030
434 3 CONTINUE 00008040
C DETERMINE THE FOURIER COEFFICIENTS FOR THE ORTHOGONALIZED MATRIX 00008050
435 C(K)=SC3/SC2(K) 00008060
436 K=K+1 00008070
437 IF (N.EQ.2) GO TO 34 00008080
438 IF (K.LT.3) GO TO 10 00008090
C DETERMINE THE ALPHA'S USED IN COMPUTING THE COEFFICIENTS OF PHI 00008100

```

```

439      JK=K-1                                00008110
440      9 JL=K                                00008120
441      JK=JK-1                              00008130
442      JJ=K-JK-1                            00008140
443      DO 8 LM=1,JJ                          00008150
444      JL=JL-1                              00008160
445      E ALPHA(JK,K)=ALPHA(JK,K)+ALPHA(JK,JL)*ALPHA(K,JL) 00008170
446      IF(JK.NE.1) GO TO 9                    00008180
447      IF(K.LT.M) GO TO 10                   00008190
C DETERMINE THE LAST FOURIER COEFFICIENT      00008200
448      34 SC2(K)=C.DC                         00008210
449      SC3=0.DC                               00008220

450      DO 7 I=1,N                            00008230
451      F=PHI(I,K)                             00008250
452      K1=K-1                                 00008260
453      DO 1 J=1,K1                            00008270
454      1 F=P+ALPHA(J,K)*PHI(I,J)             00008280
455      SC2(K)=SC2(K)+W(I)*P**2              00008290
456      7 SC3=SC3+F(I)*W(I)*P                00008300
457      C(K)=SC3/SC2(K)                       00008310
C DETERMINE THE COEFFICIENTS OF PHI          00008320
458      IDEKT=1                               00008330
459      ICOUNT=C                               00008340
460      1000 CONTINUE                          00008350
461      DO 13 I=1,M                            00008360
462      D(I)=C(I)                              00008370
463      IF(I.EQ.M) GO TO 13                   00008380
464      II=I+1                                 00008390
465      DO 14 J=II,M                           00008400
466      14 D(I)=D(I)+ALPHA(I,J)*C(J)          00008410
467      13 CONTINUE                           00008420
C COMPUTE THE VARIANCE OF THE FOURIER COEFFICIENTS AND THE VARIANCE-COVA
C MATRIX OF THE COEFFICIENTS                00008430
468      DO 15 I=1,M                            00008440
469      DO 15 J=1,M                            00008450
470      15 SUMD(I,J)=0.D0                      00008460
471      SC4=0.DC                               00008470
472      DO 22 I=1,M                            00008480
473      FN=0.DC                                00008490
474      DO 21 J=1,M                            00008510
475      21 FN=FN+D(J)*PHI(I,J)                00008520
476      V(I)=F(I)-FN                           00008530
477      V2=V(I)**2                             00008540
478      22 SC4=SC4+V2*W(I)                    00008550
479      SIGMAF=SC4/(N-M+ICOUNT)*SIGMA          00008560
480      VFC=SIGMAF                              00008570
481      IF(IDEKT.EQ.2) VFC=SC4/(N-NPC)*SIGMA  00008580
482      IF(INDEX.EQ.0) NPC=M                    00008590
483      DO 28 I=1,M                            00008600
484      SUMC(I)=SIGMAF/SC2(I)                  00008610
485      IF(IDEKT.EQ.1) GO TO 28                00008630
486      IF(C(I).EQ.0D0) SUMC(I)=0D0           00008640
487      28 CONTINUE                            00008650
488      DO 23 I=1,M                            00008660
489      DO 23 J=1,I                            00008670
490      DO 23 K=J,I                            00008680
491      23 SUMD(J,K)=SUMD(J,K)+ALPHA(J,I)*ALPHA(K,I)*SUMC(I) 00008690
492      DO 24 I=1,M                            00008700
493      II=I+1                                 00008710

```

```

494      IF (IT.GT.M) GO TO 30                                00008720
495      EC 24 J=IT,M                                         00008730
496      24 SUMC(J,I)=SUMC(I,J)                               00008740
497      30 CONTINUE                                         00008750
      C OPTIONAL CHECK FOR STATISTICALLY SIGNIFICANT FOURIER COEFFICIENTS 00008750
498      IF (INCLX.EQ.0) GO TO 40                             00008770
499      IF (IDEX).EQ.2) GO TO 40                             00008780
500      PINDEX=CFLLAT(INDEX)                                 00008790
501      EC 31 I=1,M                                          00008800
502      STDF(I)=F(INCLX*DSGRT(SUMC(I)))                      00008810
503      IF (DABS(C(I)).LT.STDF(I)) GO TO 32                  00008820
504      GO TO 31                                             00008830
505      32 C(I)=CDC                                          00008840

506      ICDUNT=ICDUNT+1                                     00008850
507      SUMC(I)=CDC                                         00008860
508      31 CONTINUE                                         00008870
509      NFC=0                                                00008880
510      EC 33 I=1,M                                          00008890
511      IF (C(I).NE.CD0) NFC=I                              00008900
512      33 CONTINUE                                         00008910
513      IDEKT=2                                             00008920
514      GO TO 1000                                          00008930
515      40 RETURN                                           00008940
516      100 WRITE(IW,102)                                    00008950
517      102 FORMAT('C', '*ERROR*  NEGATIVE DEGREES OF FREEDOM') 00008960
518      RETURN                                              00008970
519      END                                                  00008980

```

360

| NO   | LATITUDE  | LONGITUDE  | RANGE RATIO | TIME LAG   |
|------|-----------|------------|-------------|------------|
| 1000 | 45.083330 | -65.583330 | 0.000000    | 0.000000   |
| 05   | 45.266670 | -66.066670 | 1.000000    | 0.000000   |
| 1    | 44.600000 | -66.800000 | 0.661400    | -28.500000 |
| 15   | 44.883330 | -66.950000 | 0.673300    | 5.000000   |
| 40   | 45.066670 | -67.050000 | 0.500400    | 15.500000  |
| 60   | 45.233330 | -66.050000 | 0.996000    | -10.000000 |
| 120  | 45.350000 | -65.533330 | 1.201200    | 9.000000   |
| 140  | 45.516670 | -64.966670 | 1.324700    | 19.000000  |
| 150  | 45.600000 | -64.783330 | 1.410400    | 17.000000  |
| 225  | 45.466670 | -64.850000 | 1.316700    | 11.000000  |
| 235  | 45.350000 | -64.816670 | 1.310700    | -1.000000  |
| 240  | 45.300000 | -64.783330 | 1.456200    | 16.500000  |
| 305  | 45.233330 | -64.516670 | 1.490000    | 12.000000  |
| 312  | 45.230000 | -65.000000 | 1.630600    | 0.000000   |
| 315  | 45.050000 | -65.066670 | 1.204900    | 12.000000  |
| 320  | 44.800000 | -65.533330 | 1.039800    | -14.000000 |
| 330  | 44.800000 | -65.633330 | 0.956200    | -15.500000 |
| 335  | 44.500000 | -66.100000 | 0.842600    | -18.000000 |
| 336  | 44.783330 | -66.000000 | 0.761000    | -37.000000 |
| 340  | 44.266670 | -66.350000 | 0.721100    | -34.000000 |
| 345  | 44.250000 | -66.400000 | 0.713100    | -34.000000 |
| 353  | 44.333330 | -66.116670 | 0.721100    | 18.000000  |
| 355  | 44.200000 | -66.166670 | 0.673300    | 18.000000  |

DEG. OF POLY.      NL. OF OBS.      NO. OF COEFF.      DEG. OF FREEDOM

## II.2 Least Squares Polynomial Approximation of Observed Time Series

Figure A-2 is the flow chart describing the program. The program uses any number of constituent frequencies - ICON and the required number of coefficients is computed from

$$U = 2*ICON + 1 .$$

### INPUTS

1st card: FORMAT Free, contains the following:

M - number of observations

ICON - number of constituent frequencies

ITEST - code for testing Fourier Coefficients

0 - for no test

1 - test against its Standard deviation

2 - test against 2 times its Standard deviation

3 - test against 3 times its standard deviation

ICON cards: FORMAT(10X, F15.6)

Each card contains one constituent frequency.

ICON cards: FORMAT(5X, F10.6, 5X, F10.6)

Each card contains the nodal (modulation) factor and the astronomical arguments required if harmonic constants are to be computed. If harmonic constants are not being computed, these cards should be omitted.

M cards: FORMAT(5X, F10.3, 5X, F10.3),

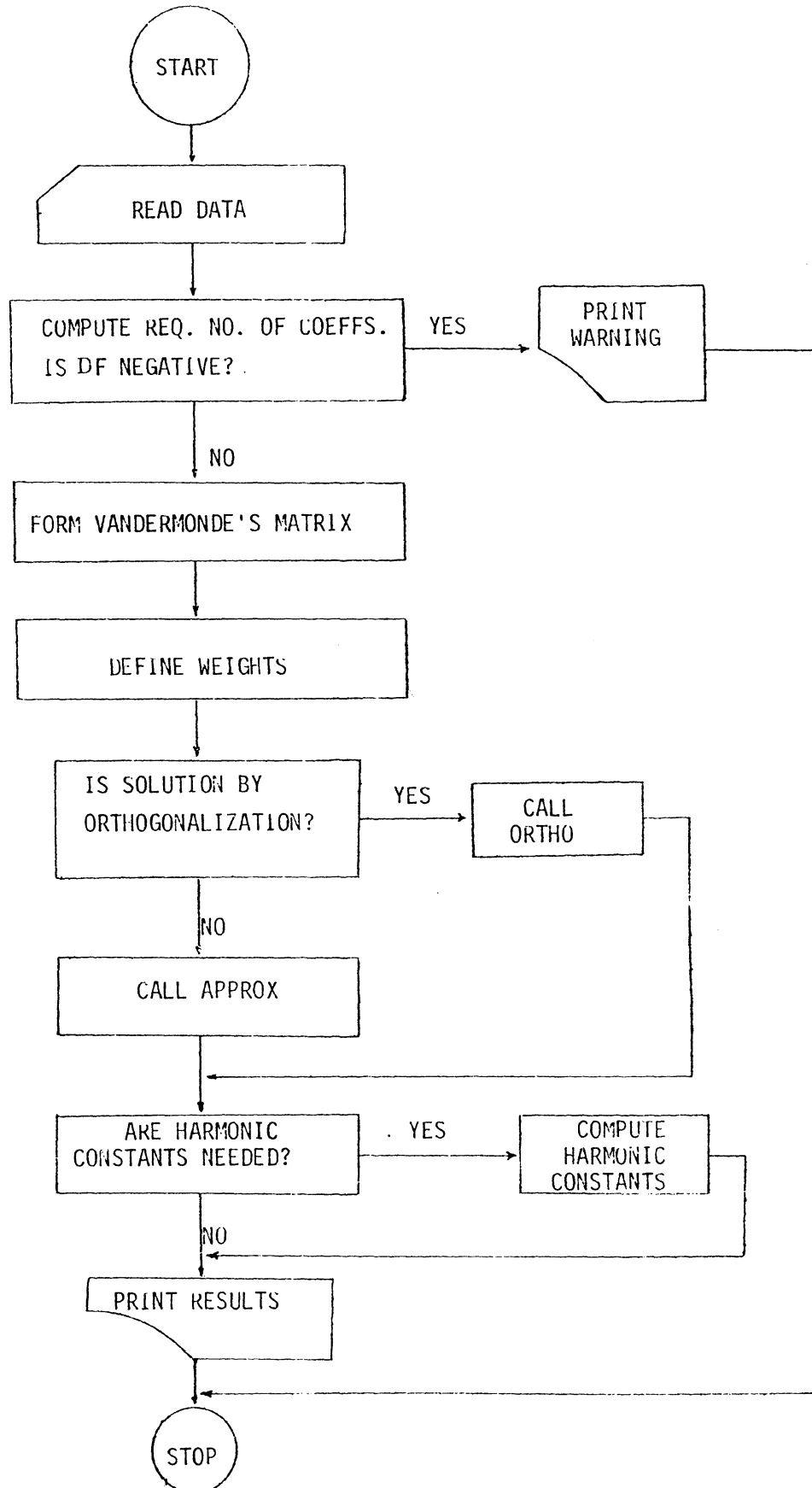
each card contains the observed height and the time of observation.

## SUBROUTINES:

The Subroutines used are APPROX and ORTHO as in the previous case.



Figure II-2 Polynomial Approximation of Observed Time Series - Flow Chart





```

12          LU 20 I=1, ICCN
13          HEAD(5,112)W(I)

14          112          FORMAT(10X,F15.6)
15          20          CONTINUE
16          UC 32 I=1, ICCN
17          HEAD(5,121)FK(I),VK(I)
18          121          FCFMAT(SX,F10.6,SX,F10.6)
19          VK(I)=VK(I)*PI/180.
20          32          CONTINUE

C
C          READ IN THE OBSERVED TIME AND HEIGHT. NOTE THE ORIGIN FOR TIME
C          CAN BE TAKEN AS ZERO HOUR OF THE FIRST DAY OF OBSERVATION--
C          THE TIME IS IN HOURS.
C

21          DC 21 I=1,M
22          HEAD(5,113)FH(I),T(I)
23          113          FCFMAT(SX,F10.4,SX,F10.3)
24          21          CONTINUE
25          ERROR=0.05

C
C          PRINT OUT THE INPUT DATA
C

26          PRINT,'*** CONSTITUENT FREQUENCIES *****'
27          DC 22 I=1, ICCN
28          PRINT112,W(I)
29          22          CONTINUE
30          PRINT114
31          114          FCFMAT(/,SX,'HEIGHT OBSERVED',SX,'TIME OF OBS. ')
32          DC 23 I=1,M
33          PRINT124,FH(I),T(I)
34          124          FCFMAT(SX,F10.4,10X,F10.3)
35          23          CONTINUE

C
C          CONVERT FREQUENCIES TO RADIAN MEASURE
36          DC 25 K=1, ICCN
37          W(K)=W(K)*PI/180.
38          25          CONTINUE

C
C          FORMATION OF VANDERMOND MATRIX
C

39          DO 24 I=1,M
40          DO 24 J=1,L
41          A(I,J)=0.000
42          24          CONTINUE
43          DC 25 I=1,M
44          J=1
45          A(I,J)=1.000
46          K=C
47          DO 27 J=2,L,2
48          K=K+1
49          A(I,J)=COS(W(K)*T(I))
50          27          CONTINUE
51          K=C
52          DO 28 J=3,L,2
53          K=K+1
54          A(I,J)=SIN(W(K)*T(I))
55          28          CONTINUE
56          25          CONTINUE

```

```

C PRINT THE MATRIX A
C
57          PRINT, '*** VANDEMOND MATRIX ***'
C
58          CALL MOUTE(A, IDA, M, L)
C
59          IF (ID.EQ.1) GO TO 35
C
C          FORMATION OF WEIGHT MATRIX
C
60          DO 30 I=1, M
61            DO 30 J=1, M
62              F(I, J)=0.000
63            CONTINUE
64          DO 31 I=1, M
65            J=I
66            F(I, J)=1./ERRFOR**2
67          CONTINUE
C
C          PERFORM THE LEAST SQUARES APPROX. BY CALLING THE SUBROUTINE
C          APPRFX
C
68          CALL APPRFX(L, M, A, P, FH, EN, C, V, AC, U, COVAR, APVF)
69          PRINT116
70          FORMAT(//, 10X, 'VECTOR OF COEFFICIENTS')
71          CALL MOUTE(C, IDA, L, 1)
72          PRINT117
73          FORMAT(//, 10X, 'RESIDUAL', 10X, 'VECTOR')
74          CALL MOUTE(V, IDA, M, 1)
75          PRINT, 'NUMBER OF DEGREES OF FREEDOM=', IDF
76          PRINT118, APVF
77          FORMAT(//, 10X, 'A POSTERIORI VARIANCE FACTOR =', F10.6)
78          PRINT119
79          FORMAT(//, 10X, 'VARIANCE COVARIANCE MATRIX OF THE COEFF. ')
80          CALL MOUTE(COVAR, IDA, L, L)
81          IF (ID.EQ.2) GO TO 40
82          CONTINUE
83          DO 36 I=1, M
84            WT(I)=1./ERRFOR**2
85          CONTINUE
86          CALL CRIFC(M, L, APW, A, IDA, SIGM, VFC, NPC, ITEST, V, COVAR, FH,
*          WT, C, ALPHA, FC, SUMFC, SC2, STDP, IW)
87          PRINT125
88          DO 37 I=1, L
89            PRINT126, FC(I), SUMFC(I)
90          CONTINUE
91          PRINT, 'FOURIER COEFF. -A POSTERIORI V.F.= ', SIGM
92          PRINT, 'NO OF ORIGINAL COEFF. AFTER TEST= ', NPC
93          PRINT127
94          DO 38 I=1, L
95            PRINT128, C(I)
96          CONTINUE
97          PRINT129
98          CALL MOUTE(COVAR, IDA, L, L)
99          PRINT130
100         DO 39 I=1, M
101           PRINT128, V(I)
102         CONTINUE
103         PRINT, 'A POSTERIORI V.F. ORIGINAL POLYNOMIAL =', VFC

```

C PREDICTION OF HEIGHTS USING THE POLYNOMIAL FOR SOME TIME INTERVALS

```

104      WRITE(6,132)
105      WRITE(6,133)

106      DC 45 I=1,50
107      TP=I
108      PHI(1)=1.0
109      DC 46 K=1,ICCN
110      JC=2*K
111      JS=2*K+1
112      PHI(JC)=DCOS(W(K)*TP)
113      PHI(JS)=DSIN(W(K)*TP)
114      46  CONTINUE
115      SUM=0
116      SUMVA=)
117      DC 47 IF=1,L
118      FH=C(IP)*PHI(IP)
119      PV=PHI(IP)**2*CCVAR(IP,IP)
120      SUM=SUM+FH
121      SUMVA=SUMVA+PV
122      47  CONTINUE
123      STD=DSQRT(SUMVA)
124      WRITE(6,131)TP,SUM,STD
125      45  CONTINUE
126      DC 48 I=1,L
127      WRITE(7,42)C(I),COVAR(I,I)
128      48  CONTINUE
129      42  FORMAT(5X,E11.4,5X,E11.4)
130      132  FORMAT('1',15X,'HOURLY PREDICTIONS IN THE TIME INTERVAL')
131      133  FORMAT(//,5X,'TIME IN HOURS',5X,'PR. HEIGHTS',5X,'STD. ERR. OF
*PR. HEIGHTS')
132      131  FORMAT(//,8X,F6.2,9X,F6.3,12X,F10.5)
133      125  FORMAT(//,5X,'FOURIER COEFFS.',10X,'VARIANCES')
134      126  FORMAT(//,5X,E11.4,10X,E11.4)
135      127  FORMAT(//,5X,'VECTOR OF ORIGINAL COEFFICIENTS')
136      128  FORMAT(//,5X,E11.4)
137      129  FORMAT(//,5X,'VARIANCE COVARIANCE MATRIX OF ORIGINAL COEFF.')
138      130  FORMAT(//,5X,'VECTOR OF RESIDUALS')
139      40  IF(NHC.EC.0)GO TO 41
140          DC 33 I=2,L,2
141          J=I+1
142          IA=I/2
143          PHASE(IA)=DATAN2(C(J),C(I))
144          E(IA)=1./(FK(IA)*DCOS(PHASE(IA)))
145          FK(IA)=C(I)*B(IA)
146          VKF(IA)=2.*FI-VKR(IA)
147          XKAF(IA)=VKR(IA)-PHASE(IA)
148          EP(1,1)=(1./(1.+(C(J)/C(I))**2))*(-1*C(J)/C(I)**2)
149          EP(1,2)=(1./(1.+(C(J)/C(I))**2))*1./C(I)
150          SIGMAF(IA)=EP(1,1)**2*CCVAR(I,1)+EP(1,2)**2*COVAR(J,J)
151          EA(1,1)=1./FK(IA)*DCOS(PHASE(IA))
152          EA(1,2)=C(I)*DSIN(PHASE(IA))/(FK(IA)*DCOS(PHASE(IA))**2)
153          SIGMAA(IA)=EA(1,1)**2*CCVAR(I,1)+EA(1,2)**2*SIGMAF(IA)
154      33  CONTINUE
155      PRINT122
156      122  FORMAT(//,5X,'CONSTITUENT',5X,'AMPLITUDE',5X,'SIGMA AMPL.',10X,
* 'PHASE LAG',5X,'SIGMA PHASE')
157      DC 34 K=1,ICCN

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```

159          XKAPA(K)=XKAPA(K)*180./PI
160          SIGMAF(K)=DSQRT(SIGMAF(K))*180./PI
161          SIGMAA(K)=DSQRT(SIGMAA(K))
162          PRINT123,W(K),FK(K),SIGMAA(K),XKAPA(K),SIGMAF(K)
163          FORMAT(/,5X,F12.6,3X,F10.6,5X,E11.4,10X,F12.6,5X,E11.4)

164          34          CONTINUE
165          41          CONTINUE
166          STOP
167          END
C

168          SUBROUTINE APPROX(L,M,A,P,F,EN,C,V,AC,U,COVAR,APVF)
C
C          *****
C          *          THIS SUBROUTINE DOES THE LEAST SQUARES APPROXIMATION *
C          *          OF THE GIVEN FUNCTIONS AND RETURNS THE VECTOR OF COEF *
C          *          FICIENTS TOGETHER WITH THE VAR. COVAR. MATRIX *
C          *****
C
169          IMPLICIT REAL*8(A-F,L-2)
170          DIMENSION A(60,60),P(60,60),EN(60,60),AC(60),C(60),F(60),
          *          COVAR(60,60),ATP(60,60),AT(60,60),VTP(1,60),VF(1),U(60),
          *          IW1(60),IW2(60),VT(1,60),V(60,1)
171          IRDA=ICA=ICE=IDC=60
172          CALL TRNSD(AT,ICE,A,IDA,M,L)
173          CALL MMULD(ATP,IDC,AT,ICA,P,IDB,L,M,M)
174          CALL MMULD(EN,IDC,ATP,ICA,A,IDE,L,M,L)
175          CALL MMULD(U,IDC,ATP,IDA,F,IDB,L,M,1)
176          CALL MINVD(EN,IRDA,L,DETA,IW1,IW2)
177          CALL MMULD(C,IDC,EN,IDA,U,IDE,L,L,1)
C          COMPLETE RESIDUALS
178          CALL MMULD(AC,IDC,A,IDA,C,IDC,M,L,1)
179          CALL MSUED(V,IDC,AC,IDA,F,IDC,M,1)
C          COMPLETE A POSTERIORI VARIANCE FACTOR
180          CALL TRNSD(VI,1,V,IDA,M,1)
181          CALL MMULD(VTP,1,VT,1,P,IDB,1,M,M)
182          CALL MMULD(VF,1,VTP,1,V,IDB,1,M,1)
183          IDF=M-L
184          APVF=VF(1)/IDF
C          COMPUTE VARIANCE COVARIANCE MATRIX OF COEFFICIENT
185          DO 10 I=1,L
186          DO 10 J=1,L
187          COVAR(I,J)=APVF*EN(I,J)
188          10          CONTINUE
189          10          RETURN
190          END
C

191          SUBROUTINE CRTFC(N,M,SIGMA,PHI,MRD,SIGMAF,VFC,NPC,INDEX,V,SUMD,F,W0007370
          &.D,ALPHA,          C,SUMC,SCR,STDP,IW)          00007380
C
C          THIS SUBROUTINE ORTHOGONALIZES THE MATRIX PHI USING THE GRAM-SCHMIDT 00007390
C          METHOD, COMPUTES THE FOURIER COEFFICIENTS OF THE ORTHOGONALIZED MATRIX 00007400
C          DERIVES THE COEFFICIENTS OF PHI, COMPUTES THE VARIANCES OF THE FOURIER 00007410
C          COEFFICIENTS AND THE VARIANCE-COVARIANCE MATRIX OF THE COEFFICIENTS 00007420
C          INPUTS :          00007430
C          1. PHI(OPTIONAL - COULD BE FUNCTION SUBPROGRAM INSTEAD) - AN N BY M 00007440
C          CONTAINING THE BASE FUNCTIONS EVALUATED FOR EACH OBSERVATION 00007450
C          2. N - THE NUMBER OF OBSERVATIONS          00007460

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C      3. N - THE NUMBER OF BASE FUNCTIONS (EQUAL OR GREATER THAN 2)          00007470
C      4. W - A VECTOR OF LENGTH N CONTAINING THE COMPUTED WEIGHT FUNCTIONS 00007480
C      5. F - FUNCTIONAL VALUES                                             00007490
C      6. SIGMA - THE A PRIORI VARIANCE FACTOR                               00007500
C      7. NRD - THE MAXIMUM ROW DIMENSION OF PHI                            00007510
C      8. MRC - THE MAXIMUM COLUMN DIMENSION OF PHI                          00007520

C      9. INDEX - PERMITS OPTIONAL TEST FOR STATISTICAL SIGNIFICANCE        00007530
C      OF FOURIER COEFFICIENTS....                                          00007540
C      IF 0, STATISTICAL TEST FOR FOURIER COEFFICIENTS ABANDONED            00007550
C      IF 1, TESTS AGAINST ONE TIME ITS STANDARD DEVIATION                  00007560
C      IF 2, TESTS AGAINST TWICE ITS ST. DEVIATION                          00007570
C      IF 3, TESTS AGAINST THREE TIMES ITS ST. DEVIATION                    00007580
192  C      IMPLICIT REAL*(A-H,O-Z)                                           00007750
C      10. IW - WRITE CODE OF THE COMPUTER                                   00007590
C      OUTPUTS :                                                            00007600
C      1. ALPHA - AN MRC BY M MATRIX CONTAINING THE ALPHA'S USED IN COMPUTIO 00007610
C      THE ORTHOGONALIZED MATRIX AND IN COMPUTING THE COEFFICIENTS OF PHO 00007620
C      2. C - THE M FOURIER COEFFICIENTS OF THE ORTHOGONALIZED MATRIX        00007630
C      3. D - THE M COEFFICIENTS OF THE INPUT MATRIX PHI                     00007640
C      4. SUMC - THE VARIANCES OF THE FOURIER COEFFICIENTS                  00007650
C      5. SUMD - THE VARIANCE-COVARIANCE MATRIX OF THE COEFFICIENTS         00007660
C      6. SC2 - THE SQUARES OF THE NORMS OF THE ORTHOGONALIZED MATRIX      00007670
C      7. SIGMAF - THE FOURIER POLYNOMIAL A POSTERIORI VARIANCE FACTOR     00007680
C      8. V - THE N RESIDUALS                                               00007690
C      9. VFC - THE ORIGINAL POLYNOMIAL A POSTERIORI VARIANCE FACTOR       00007700
C      10. NPC - NUMBER OF THE COEFFICIENTS OF THE ORIGINAL POLYNOMIAL     00007710
C      AFTER THE STATISTICAL TEST IS PERFORMED                             00007720
C      11. STDP - VECTOR AGAINST WHICH THE ABSOLUTE VALUES OF FOURIER     00007730
C      COEFFICIENTS ARE TESTED                                             00007740
193  C      DIMENSION ALPHA(60,60),W(60),F(60),C(60),D(60),SUMC(60),
C      * SUMD(60,60),SC2(60),V(60),STDP(60),PHI(60,60)
C      TEST FOR NEGATIVE DEGREES OF FREEDOM                                00007800
194  C      IF (N.LT.M) GO TO 100                                             00007810
195  C      K=1                                                                00007820
196  C      ALPHA(M,M)=1.00                                                  00007830
C      DETERMINE THE ALPHA'S FOR COMPUTATION OF ORTHOGONALIZED MATRIX      00007840
197  C      10 DO 3 J=K,M                                                    00007850
198  C      IF (J.NE.K) GO TO 6                                              00007860
199  C      ALPHA(K,K)=1.00                                                  00007870
200  C      GO TO 3                                                         00007880
201  C      6 SC1=0.00                                                       00007890
202  C      SC2(K)=C.00                                                      00007900
203  C      SC3=0.00                                                         00007910
204  C      DO 2 I=1,N                                                       00007920
205  C      P=PHI(I,K)                                                       00007940
206  C      IF (K.EQ.1) GO TO 4                                             00007950
207  C      K1=K-1                                                           00007960
208  C      DO 5 J1=1,K1                                                     00007970
209  C      F=P*ALPHA(J1,K)*PHI(I,J1)                                       00007980
210  C      4 SC1=SC1+W(I)*PHI(I,J)*P                                       00007990
211  C      SC3=SC3+F(I)*W(I)*P                                             00008000
212  C      2 SC2(K)=SC2(K)+W(I)*P**2                                       00008010
213  C      ALPHA(J,K)=-SC1/SC2(K)                                          00008020
214  C      ALPHA(K,J)=ALPHA(J,K)                                          00008030
215  C      3 CONTINUE                                                       00008040
C      DETERMINE THE FOURIER COEFFICIENTS FOR THE ORTHOGONALIZED MATRIX    00008050
216  C      C(K)=SC3/SC2(K)                                                 00008060
217  C      K=K+1                                                            00008070
218  C      IF (M.EQ.2) GO TO 34                                           00008080

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219      IF(K.LT.3) GO TO 10          00008090
      C DETERMINE THE ALPHA'S USED IN COMPUTING THE COEFFICIENTS OF PHI  00008100
220      JK=K-1                      00008110
221      5 JL=K                       00008120
222      JK=JK-1                     00008130
223      JJ=K-JK-1                   00008140
224      DO 8 LM=1, JJ                00008150

225      JL=JL-1                      00008160
226      8 ALPHA(JK,K)=ALPHA(JK,K)+ALPHA(JK,JL)*ALPHA(K,JL) 00008170
227      IF(JK.NE.1) GO TO 5          00008180
228      IF(K.LT.M) GO TO 10          00008190
      C DETERMINE THE LAST FOURIER COEFFICIENT 00008200
229      34 SC2(K)=0.DC                00008210
230      SC3=0.DC                     00008220
231      DO 7 I=1,N                   00008230
232      F=PHI(I,K)                   00008250
233      K1=K-1                       00008260
234      DO 1 J=1,K1                  00008270
235      1 P=P+ALPHA(J,K)*PHI(I,J)    00008280
236      SC2(K)=SC2(K)+W(I)*P**2      00008290
237      7 SC3=SC3+F(I)*W(I)*F       00008300
238      C(K)=SC3/SC2(K)              00008310
      C DETERMINE THE COEFFICIENTS OF PHI 00008320
239      IDEKT=1                       00008330
240      ICCUNT=C                      00008340
241      1000 CONTINUE                 00008350
242      DO 13 I=1,M                   00008360
243      C(I)=C(I)                     00008370
244      IF(I.EQ.M) GO TO 13           00008380
245      II=I+1                        00008390
246      DO 14 J=II,M                 00008400
247      14 C(I)=C(I)+ALPHA(I,J)*C(J)  00008410
248      13 CONTINUE                   00008420
      C COMPUTE THE VARIANCE OF THE FOURIER COEFFICIENTS AND THE VARIANCE-COVA 00008430
      C MATRIX OF THE COEFFICIENTS 00008440
249      DO 15 I=1,M                   00008450
250      DO 16 J=1,M                   00008460
251      15 SUMC(I,J)=0.D0              00008470
252      SC4=0.DC                       00008480
253      DO 22 I=1,N                   00008490
254      FN=0.D0                        00008510
255      DO 21 J=1,M                   00008520
256      21 FN=FN+D(J)*PHI(I,J)        00008530
257      V(I)=F(I)-FN                  00008540
258      V2=V(I)**2                    00008550
259      22 SC4=SC4+V2*W(I)             00008560
260      SIGMAF=SC4/(N-M+ICCUNT)*SIGMA  00008570
261      VFC=SIGMAF                     00008580
262      IF(IDEKT.EQ.2) VFC=SC4/(N-NFC)*SIGMA 00008590
263      IF(INDEX.EQ.0) NPC=N           00008600
264      DO 28 I=1,M                   00008610
265      SUMC(I)=SIGMAF/SC2(I)          00008630
266      IF(IDEKT.EQ.1) GO TO 28       00008640
267      IF(C(I).EQ.CDC) SLMC(I)=0DC   00008650
268      28 CONTINUE                   00008660
269      DO 23 I=1,M                   00008660
270      DO 23 J=1,I                   00008670
271      DO 23 K=J,I                   00008680
272      23 SUMD(J,K)=SUMD(J,K)+ALPHA(J,I)*ALPHA(K,I)*SUMC(I) 00008690

```



```

273      DC 24 I=1,M                      00008700
274      IT=I+1                          00008710
275      IF(11.GT.M) GC TO 30            00008720
276      DC 24 J=IT,M                    00008730
277      24 SUMC(J,I)=SUMC(I,J)          00008740
278      30 CONTINUE                      00008760
C OPTIONAL CHECK FOR STATISTICALLY SIGNIFICANT FOURIER COEFFICIENTS 00008750
279      IF(INDEX.EQ.0) GC TO 40          00008770

280      IF(IDEKT.EQ.2) GC TO 40          00008780
281      PINDEX=DFLCAT(INDEX)            00008790
282      DC 31 I=1,M                      00008800
283      STDP(I)=FINDEX*DSQRT(SUMC(I))    00008810
284      IF(CABS(C(I)).LT.STDP(I)) GC TO 32 00008820
285      GC TO 31                          00008830
286      32 C(I)=CDC                      00008840
287      ICDUNT=ICDUNT+1                  00008850
288      SUMC(I)=CDC                      00008860
289      31 CONTINUE                      00008870
290      NPC=0                             00008880
291      DC 33 I=1,M                      00008890
292      IF(C(I).NE.CD0) NPC=I           00008900
293      33 CONTINUE                      00008910
294      IDEKT=2                          00008920
295      GC TO 1000                        00008930
296      40 RETURN                         00008940
297      100 WRITE(IM,102)                 00008950
298      102 FORMAT('0','**ERROR*  NEGATIVE DEGREES OF FREEDOM*') 00008960
299      RETURN                            00008970
300      END                               00008980

```

```

$GU
*** CONSTITUENT FREQUENCIES *****
28.584104
30.000000
13.943030
15.041069
14.958531
30.082137
28.439730

```

| HEIGHT OBSERVED | TIME OF OBS. |
|-----------------|--------------|
| 7.4000          | 3.833        |
| 1.3000          | 10.250       |
| 7.3000          | 16.250       |
| 1.2000          | 22.350       |
| 7.4000          | 28.667       |
| 1.2000          | 35.000       |
| 7.3000          | 41.250       |
| 1.2000          | 47.250       |
| 7.6000          | 53.500       |
| 1.2000          | 59.917       |
| 7.3000          | 66.083       |
| 1.1000          | 72.417       |
| 7.7000          | 78.583       |
| 1.0000          | 84.833       |
| 7.4000          | 91.083       |
| 1.1000          | 97.417       |

### II.3 Tidal Reductions

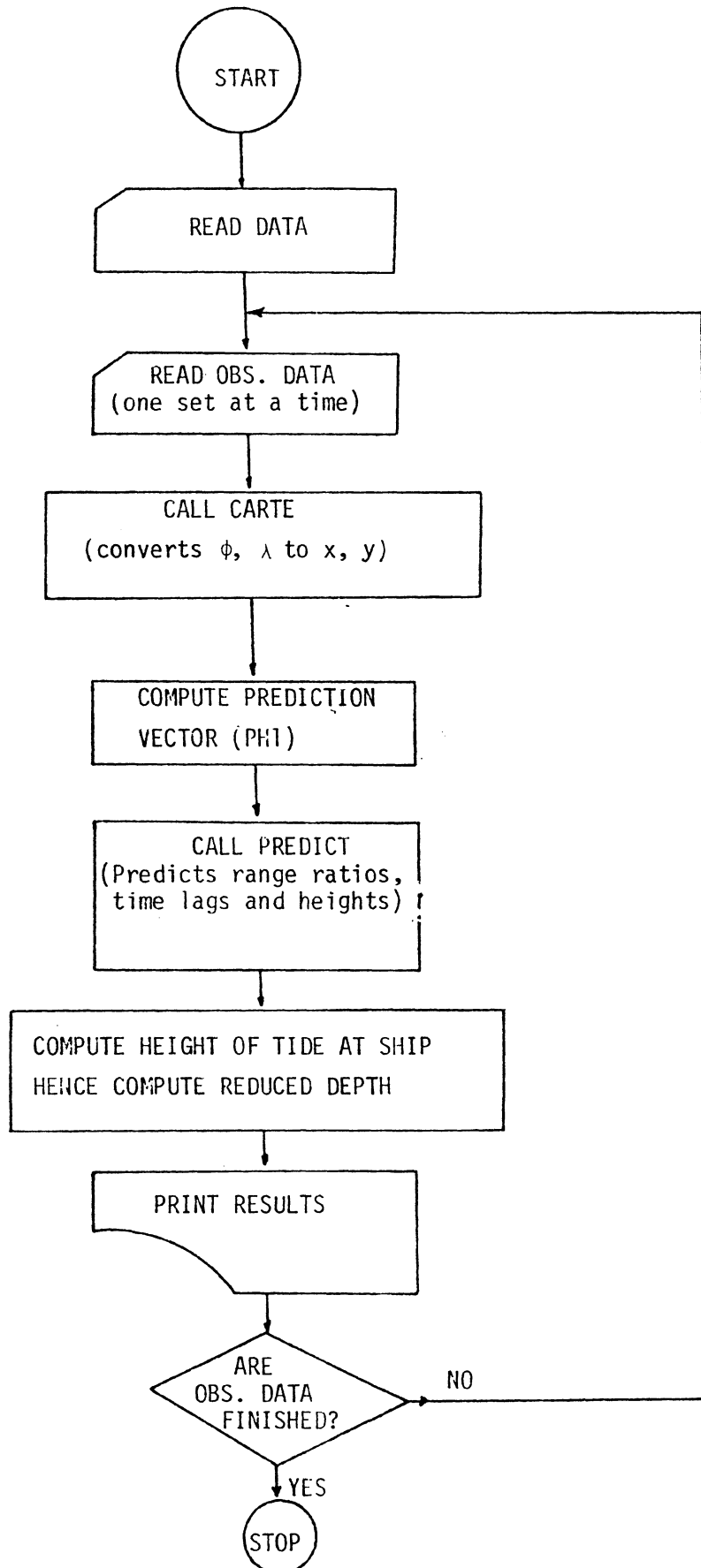
Figure A-3 is the flow chart describing the tidal reduction computations.

The program uses as input the following:

- the results of computations 1 and 2, that is; the coefficients of the approximating polynomials  $C_r$ ,  $C_A$  and  $C_B$  and their associated standard deviations.
- the observed data at each sounding namely: the depth sounded (D), time of sounding (t) and the geodetic coordinates ( $\phi$ ,  $\lambda$ ) or the local Cartesian coordinates (x, y). The observed data at each sounding are punched in one card and read into the computer one card at a time.

The SUBROUTINE PREDICT used in this program is different from the SUBROUTINE PRED. The subroutine predict uses prediction vector and predicts for one point at a time while the Subroutine Pred uses prediction matrix and predicts for all the points at once.

Figure II-3 Tidal Reductions - Flow Chart



\*\*\*\*\*FABL

JOB CKENWA/G, TIME=8, PAGES=10

PROGRAM TO IMPLIMENT TIDAL REDUCTIONS USING ANALYTICAL COTIDAL CHART IN REAL TIME.

N O T A T I O N S
N=DEGREE OF POLY. FOR RANGE RATIO
L=NUM.OF COEFFS. IN THE POLY.
LT=NUM.OF COEFFS. IN THE POLY.OF THE TIME SERIES AT THE REF. STATION
NCCN= NUM OF CONSTITUENT FREQ USED IN REAL TIME APPROX AT REF. STN.
ITYPE= THE TYPE OF INFORMATION
1- FOR LAT. LONG.GIVEN
2- FOR X,Y COORDS.GIVEN
ISPLIT = CODE INDICATING WETHER THE TIME LAG IS SPLIT INTO FUNCTION A AND B
1 - FOR NO SPLIT
2 - FOR SPLIT
CT= POLY. COEFFS.FOR REAL TIME AT THE REF STN.
CR=POLY. COEFFS.FOR RANGE RATIO
VACT,VACR= ASSOCIATED VARIANCES FOR CT,CR RESPECTIVELY
CA,CB=COEFFS.OF POLY. FOR TIME LAG
VACA,VACB=ASSOCIATED VARIANCES FOR CA CB RESPECTIVELY
DECLA0,XLAMO,SECLA0 = LATITUDE OF REF. STN. IN DEGREE,MIN. AND SECONDS
DEGL00,XL0M0,SECL00 = LONGITUDE OF REF. STN.IN DEGREES MIN. AND SECONDS
T,XMIN = OBSERVED TIME IN HOURS AND MINS.
DEPTH0 = OBSERVED DEPTH IN METRES
DEGLA,XLAMIN,SECLA = OBSERVED LATITUDE IN DEGREES,MINS,AND SECS.
DEGLC,XLCMIN,SECLU = OBSERVED LONGITUDE IN DEGREES.MINS.AND SECS.

\*\*\*\*\*

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```
1 IMPLICIT REAL*8(A-H,O-Z)
2 DIMENSION CT(30),CR(30),PHIT(30),PHIR(30),CA(30),CB(30),
* VACT(30),VACR(30),VACA(30),VACB(30),H(300),STE(300),PHI(300),
* ALING(300),W(10),TL(300)
3 PI=3.14159265
4 CK=0.5*PI/180.
5 READ(5,40)L,LT,N,NCON,ITYPE,ISPLIT
6 DO 1 K=1,NCCN
7 READ(5,50)W(K)
8 W(K)=W(K)*PI/180.
9 CONTINUE
10 DO 6 I=1,LT
11 READ(5,45)CT(I),VACT(I)
12 CONTINUE
13 DO 7 I=1,L
14 READ(5,45)CR(I),VACR(I)
15 CONTINUE
16 DO 8 I=1,L
17 READ(5,45)CA(I),VACA(I)
18 CONTINUE
19 IF(ISPLIT.EG.1)GO TO 10
20 DO 9 I=1,L
21 READ(5,45)CE(I),VACB(I)
22 CONTINUE
23 CONTINUE
```

```

24      READ(5,44)DEGLAO,XLAMU,SECLAO,DEGLCO,XLOMU,SECLOU
25      CALL DEGRAD(DEGLAO,XLAMC,SECLAO,PHIO)
26      CALL DEGRAD(DEGLCO,XLOMO,SECLCO,ALON)
27      WRITE(6,57)
28      WRITE(6,56)
29      KLCNT=0
30      2 CONTINUE
31      IF(ITYPE.EC.1)THEN DO
32      READ(5,55)T,XMIN,DEPTHC,DEGLA,XLAMIN,SECLA,DEGLD,XLOMIN,SECLD
33      IF(T.LT.0.0)GO TO 39
34      KLCNT=KLCNT+1
35      CALL DEGRAD(DEGLA,XLAMIN,SECLA,XLAT)
36      CALL DEGRAD(DEGLD,XLCMIN,SECLD,XLCNG)
37      CALL CARTE(XLAT,XLCNG,PHIO,ALCN,X,Y)
38      ELSE DO
39      READ(5,60)T,XMIN,DEPTHC,X,Y
40      IF(T.LT.0.0)GO TO 39
41      END IF

```

```

C
C COMPUTE THE VECTOR PHI FOR PREDICTION
C

```

```

42      IDP=N+1
43      I=C
44      DO 4 K=1,IDP
45      KA=K-1
46      DO 5 J=1,IDP
47      JA=J-1
48      I=I+1
49      PHIR(I)=X**KA*Y**JA
50      5 CONTINUE
51      4 CCNTINUE
52      CALL PRDICT(L,PHIR,CR,VACR,R,STDR)
53      CALL PRDICT(L,PHIR,CA,VACA,A,STDA)
54      IF(ISPLIT.EQ.1)GO TO 11
55      CALL PRDICT(L,PHIR,CB,VACB,B,STDB)
56      TC=DATAN(B/A)/CK
57      B1=(1./(1.+(E/A)**2))*(-B/A**2)
58      B2=(1./(1.+(E/A)**2))*(1./A)
59      VATC=B1**2*STDA+B2**2*STDB
60      11 IF(ISPLIT.EC.2)GO TO 12
61      TC=A
62      VATC=STDA
63      12 CCNTINUE
64      TL(KLCNT)=TC
65      TCH=TC/60.
66      TOH=T+XMIN/60.
67      TAR=TCH-TCH

```

```

C
C COMPUTE PHI FOR THE PREDICTION OF HEIGHT AT THE REF. STATION
C

```

```

68      PHIT(1)=1.0
69      K=C
70      DO 3 I=2,LT,2
71      IA=I+1
72      K=K+1
73      PHIT(I)=CCCS(W(K)*TAR)
74      PHIT(IA)=DSIN(W(K)*TAR)
75      3 CCNTINUE
76      CALL PRDICT(LT,PHIT,CT,VACT,HTG,STDH)
77      HTIOL=HTC*R

```

```

78      STDEV=DSQRT(R**2*STDH+HTC**2*STDR)
79      DEPTH=DEFTHC-HTICE
80      STCER=DSQRT(.01**2+STDEV**2)
81      F(KCUNT)=HTICE
82      STE(KCUNT)=STDEV
83      PHI(KCUNT)=XLAT
84      ALONG(KCUNT)=XLCNG
85      XLAT=XLAT*180./PI
86      XLCNG=XLCNG*180./PI
87      WRITE(6,61)KCUNT,XLAT,XLCNG,DEPTHU,TGH,TAR,HTO,R,HTICE,STDEV
88      GO TO 2
89      39      CONTINUE
90      NOBS=KCUNT
91      WRITE(6,42)
92      DO 13 KCUNT=1,NCES
93      WRITE(6,41)KCUNT,TL(KCUNT)
94      13      CONTINUE
95      42      FORMAT('I',5X,'PREDICTED TIME LAGS')
96      41      FORMAT(/,3X,13,5X,F6.2)
97      40      FORMAT(5X,6I3)
98      50      FORMAT(5X,F10.6)
99      45      FORMAT(5X,2E11.4)
100     55      FUFMAT(5X,5F6.2)
101     60      FORMAT(5X,3F6.2,2F12.6)
102     44      FORMAT(5X,6F6.2)
103     57      FORMAT('I',//,5X,'TIDAL',5X,'REDUCTIONS')
104     56      FORMAT(/,3X,'NUM',4X,'LATITUDE',6X,'LONGITUDE',4X,'OBS. DEPTH'
      * ,3X,'TIME AT SHIP',2X,'TIME AT REF',2X,'TIDE AT REF',2X,'PR. RATIO
      * ',2X,'TIDE AT SHIP',3X,'STDEV')
105     61      FORMAT(/,3X,13,2X,F10.6,3X,F12.6,5X,F7.3,7X,F7.3,6X,F7.3,7X,F7
      * .3,4X,F7.3,6X,F7.3,2X,E11.4)
106      STOP
107      END
C
108      SUBROUTINE CARTE(ALAT,ALON,PHIO,ALONG,X,Y)
C
C      THIS SUBROUTINE COMPUTES THE CARTESIAN COORDS. FROM THE LAT.
C      AND LONGITUDE
C      *****
C
109      IMPLICIT REAL*8(A-H,C-Z)
110      RA=6378206.4D0
111      RB=6356583.8D0
112      PI=3.14159265D0
113      EC=(RA**2-RB**2)/RA**2
114      XM=((1.-EC)*RA)/(DSQRT(1.-EC*(DSIN(PHIO)**2)))**3)
115      XN=RA/DSQRT(1.-EC*(DSIN(PHIO)**2))
116      F=DSQRT(XM*XN)
117      X=R*(ALAT-PHIO)
118      Y=F*DCOS(PHIO)*(ALON-ALONO)
119      RETURN
120      END
C
121      SUBROUTINE PREDICT(L,PHI,C,VAR,PV,STDEV)
C
C      THIS SUBROUTINE PREDICTS VALUES OF THE FUNCTION USING THE COEFF.
C      OF THE POLYNOMIAL AND THE EASE FUNCTIONS.
C      *****
C

```

```

122      IMPLICIT REAL*8(A-H,O-Z)
123      DIMENSION PHI(30),C(30),VAR(30)
124      SUM=C.0
125      SUMVA=C,C
126      DO 10 I=1,L
127      FUN=PHI(I)*C(I)
128      VA=PHI(I)**2*VAR(I)
129      SUM=SUM+FUN
130      SUMVA=SUMVA+VA
131      10  CONTINUE
132      PV=SUM
133      STDEV=SUMVA
134      RETURN
135      END
C
136      SUBROUTINE DEGRAD(A,B,C,OUT)
C
C  CONVERTS DEG. MIN. SEC TO RADIANS
C
137      IMPLICIT REAL*8(A-F,U-Z)
138      OUT=(A+B/60.000+C/3600.000)*DARSIN(1.000)/90.000
139      RETURN
140      END
$GO

```

## III CANADIAN DEFINITIONS OF CHART AND SOUNDING DATUMS

This appendix has been added to supplement the information given in Sections 1.0 and 1.1 of this report. The information given here has been taken directly from the Hydrographic Tidal Manual 1970 [Energy, Mines, and Resources Canada]. The descriptions and definitions presented concern tidal waters; for similar information regarding non-tidal waters, the reader is referred to the above mentioned reference.

Chart datum is the datum plane adopted for a published chart. It is a low water datum which by international agreement is so low that the water level will seldom fall below it. It is the level above which tidal predictions and water level records are based. The datum is only used within a gauge location and differs from place to place depending on the range of tide or water level.

For tidal waters, the Canadian Hydrographic Service has adopted the level of Lower Low Water Large Tides (see Figure III-1) as its reference for chart datum, and Higher High Water Large Tides as a reference for elevations.

A sounding datum is the reference surface to which soundings are reduced during the course of a hydrographic survey. It is the datum used when compiling a "field sheet" for a survey. It may or may not be the same as chart datum.

When selecting a datum, the following must be considered:

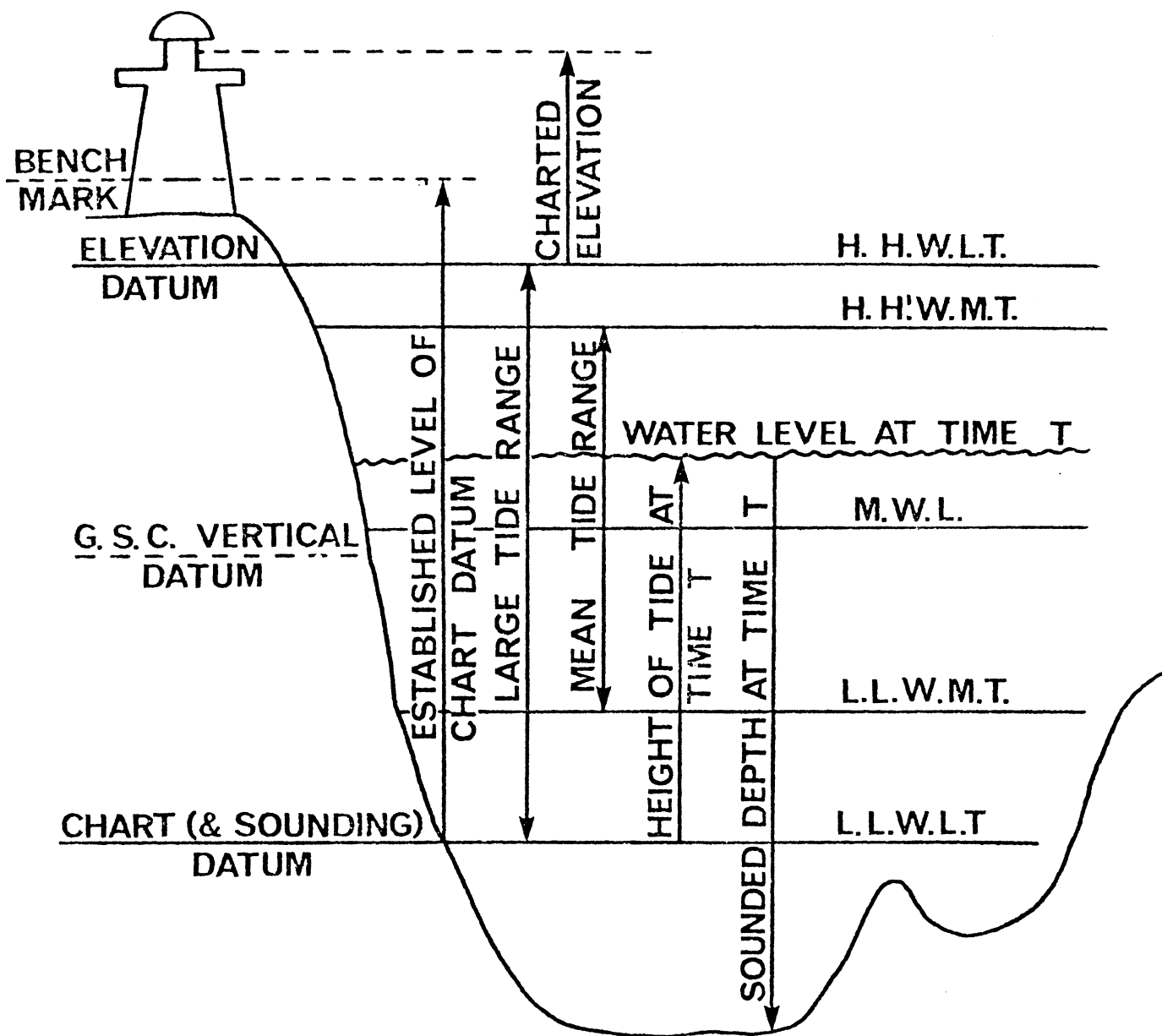
- (i) the datum should be sufficiently low so that under normal weather conditions there will always be at least the charted depth of water,
- (ii) the datum should not be so low that it gives an unduly pessimistic impression of the least depth of water likely to be found,
- (iii) the datum should be in close agreement with those of neighbouring surveys.



The following are the definitions of various reference surfaces (datum planes) and water level variations in tidal waters used by the Canadian Hydrographic Service.

Graphical representations of several of these are given in Figure III-1.

- (i) Higher High Water Large Tides (H.H.W.L.T.) is the highest predictable tide from the available tidal constituents, with the astronomical (nodal) factor  $f_k$  close to unity.
- (ii) Higher High Water Mean Tides (H.H.W.M.T.) is the mean of the predicted heights of the higher high waters of each day.
- (iii) Lower Low Water Mean Tides (L.L.W.M.T.) is the mean of the predicted heights of the lower low waters of each day.
- (iv) Lower Low Water Large Tides (L.L.W.L.T.) or Lowest Normal Tides (L.N.T.) is the lowest predictable tide from the available tidal constituents, with the astronomical (nodal) factor  $f_k$  close to unity.
- (v) Mean Water Level (M.W.L.) is the mean of hourly water levels for a period of observations.
- (vi) Mean Tide Level (M.T.L.) is the mean of all high and low water heights over a period of observation.
- (vii) Charted Elevation is the vertical distances between an object and the reference surface of Higher High Water Large Tides.
- (viii) Charted Depth is the vertical distance from the chart datum to the sea floor.



REFERENCE SURFACES AND WATER LEVEL VARIATIONS